

PRILOGA 1C

NASLOVNA STRAN NAČRTA

2.2. Načrt gradbeništva

PODATKI O GRADNJI

naziv gradnje	Prenova zdravstvene postaje Velike Lašče
kratek opis gradnje	Predmet projekta je prenova in dozidava dvigala ter stopnišča k zdravstveni postaji Velike Lašče
VRSTE GRADNJE	☐ NOVOGRADNJA - NOVOZGRAJEN OBJEKT
<i>označiti vse ustrezne vrste gradnje</i>	☒ NOVOGRADNJA - PRIZIDAVA
	☒ REKONSTRUKCIJA
	☐ SPREMEMBA NAMEMBOSTI
	☐ ODSTRANITEV CELOTNEGA OBJEKTA
	☐ LEGALIZACIJA
	☐ MANJŠA REKONSTRUKCIJA

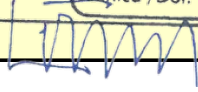
PODATKI O PROJEKTNIM DOKUMENTACIJI

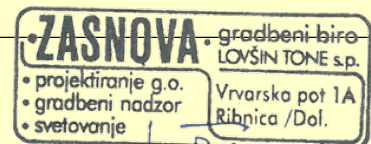
vrsta dokumentacije	PZI - projekt za izvedbo
številka projekta	ZD-01/24

PODATKI O NAČRTU

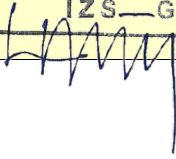
strokovno področje načrta	2 Načrt s področja gradbeništva
naziv načrta	2 Načrt gradbeništva
številka načrta	ZD-01/24-G
datum izdelave	julij 2026
datum spremembe	

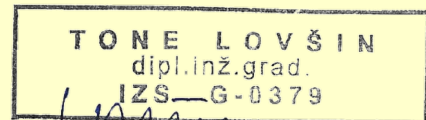
PODATKI O PROJEKTANTU NAČRTA

projektant načrta (naziv družbe)	ZASNOVA - gradbeni biro. Lovšin Tone s.p.,
naslov	Vrvarska pot 1a, 1310 Ribnica
odgovorna oseba projektanta načrta	Tone Lovšin
podpis odgovorne osebe projektanta načrta	



PODATKI O IZDELOVALCU NAČRTA

ime in priimek pooblaščenega arhitekta, pooblaščenega inženirja	Tone Lovšin dipl.inž.grad.,
identifikacijska številka	IZS G - 0379
podpis pooblaščenega arhitekta, pooblaščenega inženirja	



PRILOGA 2C

**IZJAVA PROJEKTANTA NAČRTA
IN POOBLAŠČENEGA STOKOVNJAKA,
KI JE IZDELAL NAČRT V PZI IN PID****PROJEKTANT NAČRTA**

projektant načrta (naziv družbe)	ZASNOVA - gradbeni biro. Lovšin Tone s.p.,
naslov	Vrvarska pot 1a, 1310 Ribnica
odgovorna oseba projektanta načrta	Tone Lovšin

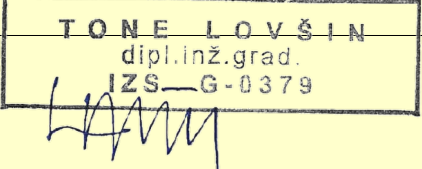
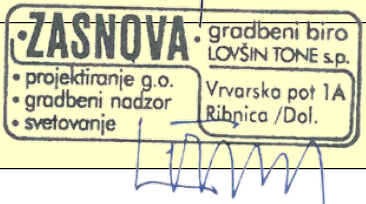
IN POOBLAŠČENI STROKOVNJAK, KI JE IZDELAL NAČRT

pooblaščen strokovnjak	Tone Lovšin dipl.inž.grad.,
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IZJAVLJAVA:**da načrt**

vrsta dokumentacije	PZI - projekt za izvedbo
strokovno področje načrta	2 Načrt s področja gradbeništva
naziv načrta	2 Načrt gradbeništva
številka načrta	ZD-01/24-G
datum izdelave	julij 2026

upošteva relevantne predpise in druge normativne dokumente ter da so upoštewane ustrezne bistvene in druge zahteve.

pooblaščen strokovnjak	Tone Lovšin dipl.inž.grad.,
identifikacijska številka	IZS G - 0379
podpis pooblaščenega strokovnjaka	
odgovorna oseba projektanta načrta	Tone Lovšin
podpis odgovorne osebe projektanta načrta	

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2.1. TEHNIČNO POROČILO

2.1.1. SPLOŠNI PODATKI

Celoten statični izračun z dimenzioniranjem nosilnih elementov je izveden skladno s Pravilnikom o mehanski odpornosti in stabilnosti objektov (Ur. List RS, št.: 101/2005) in s tem v skladu s pravili evrokodov z upoštevanjem načel.

Seznam upoštevanih standardov:

- Evrokod 0; Osnove projektiranja
- Evrokod 1: Vplivi na konstrukcije
- Evrokod 2: Projektiranje betonskih konstrukcij
- Evrokod 3: Projektiranje jeklenih konstrukcij
- Evrokod 4: Projektiranje sovprežnih konstrukcij iz jekla in betona
- Evrokod 5: Projektiranje lesenih konstrukcij
- Evrokod 6: Projektiranje zidanih konstrukcij
- Evrokod 7: Geotehnično projektiranje
- Evrokod 8: Projektiranje potresnoodpornih konstrukcij

Predmet projekta je dozidava dvigala in zunanega stopnišča k zdravstvene domu Velik Lašče. Samo stopnišče in dvigalni jašek sta izvedena iz armiranega betona. Strešna konstrukcija je izvedena kot podaljšek obstoječe strehe.

Lokacija: Velike Lašče

Nadmorska višina: cca 536,65 mNM

2.1.2. OPIS KONSTRUKCIJE

2.1.2.1. TEMELJENJE OBJEKTA

Temeljenje je izvedeno v kombinaciji, sam dvigalni jašek je temeljen na temeljni plošči – oporni zidovi so temeljeni na pasovnih temeljih.

Dvigalni jašek je izvedeno na armirano betonski plošči, katera leži na podložnem betonu in utrjenem gramoznem nasutju na geotekstilu, ki je izvedeno na homogeno podlogo vsaj 70 – 80cm pod nivojem zunanega terena. Plošča se izvede na podložni beton debeline 8 – 10 cm kvaliteta samega podložnega betona je C 12/15 (X0,CI 0,1, Dmax32, S1).

Sama talna plošča je debeline 30cm izvedena iz armiranega betona kvalitete C 25/30 (XC1,CI 0,2, Dmax 16, S4). Armatura robne palice kvalitete S-500B in mrežna armatura S-500A. Zaščitna plast minimalno 40mm.

2. NAČRT GRADBENIŠTA

Temeljna tla so predpostavili, da so srenje nosilna ocenjena dopustna nosilnost pod temeljno ploščo so ocenjene na $\sigma = 200 \text{ kN/m}^2$.

Glede na to, da bo objekt temeljen na talni plošči, bo potrebno pripraviti izkop tako, da bo po celotni površini temeljena na tampon. Tampon se vgradi po slojih debeline 20-30cm s sprotnim utrjevanjem. Tampon pod ploščo mora biti utrjen $M_s > 80 \text{ MPa}$.

Geomehansko poročilo NI bilo izvedeno.

Podporni zidovi in rob stopnišča je izvedeno na pasovnih temeljih dimenzije 60/60cm, kateri ležijo na podložnem betonu in utrjenem gramoznem nasutju na geotekstilu, ki je izvedeno na homogeno podlogo vsaj 70 – 80cm pod nivojem zunanjega terena.

Pasovni temelji so iz armiranega betona kvalitete C 25/30 (XC1, CI 0,2, $D_{\max}$ 16, S4). Armatura robne palice kvalitete B-500B. Zaščitna plast minimalno 40mm.

Med temelji se izvede talna plošča debeline 22 cm izvedena je iz armiranega betona kvalitete C 30/37 (XD3, XF4, (XA2/XA3), CI 0,2, $D_{\max}$ 16, S4). Armatura robne palice kvalitete B-500B mreže B-500A. Zaščitna plast minimalno 45mm.

Pred izvedbo potrebno preveriti ustreznost temeljnih tal z PODATKI KI SO BILI UPOŠTEVANI V STATIČNEM IZRAČUNU. V kolikor se le ti razlikujejo potrebno obvestiti projektanta.

Izkop, ter nasip mora prevzeti geomehanik z vpisom v gradbeni dnevnik, ta tudi pordi vse projektirane rešitve.

2.1.2.2. VERTIKLANE NOSILNE KONSTRUKCIJE – STENE

 **Armirano betonske stene** So debeline 20cm. Za betonažo vezi se uporabi beton SIST EN 206 C25/30 XC1 CI 0,2 $D_{\max}$ 32 S3, minimalni krovni sloj betona znaša 2,5 cm. Za armiranje se uporabi armaturno jeklo S500.

2.1.2.3. HORIZONTALNA NOSILNE KONSTRUKCIJE – LESENA KONSTRUKCIJA - ostrešja

Izvede se kot podaljšek obstoječe streh. Na koncu se naredi kot 3 kapnica. Zaradi večje dolžine so se le te zavetrjuje z lesenimi križi. Same sohe, povezniki in razpiralce so dimenzije 18/18cm. Dimenzije kapnil leg 18/26cm, vmesnih 18/26cm.

Masiven žagan les, ki se bo uporabljal v nosilnih gradbenih konstrukcijah, mora ustrezati zahtevam harmoniziranega standarda SIST EN 14081-1.

Kvaliteta lesa mora biti določena skladno s standardom, ki dovoljuje vizualno in strojno razvrščanje lesa.

Izvajalec dela mora pred pričetkom izvedbe pripraviti » Program kontrole zagotavljanja kakovosti izvedenih del«, kjer je moral upoštevati vse zahteve iz projektne dokumentacije.

2.1.3. UPORABLJENI MATERIALI

- Beton: C 25/30 $f_{ck} = 2,50 \text{ kN/cm}^2$,
 $f_{ck,cube} = 3,0 \text{ kN/cm}^2$,
 $E_{cm} = 3100 \text{ kN/cm}^2$

razred izpostavljenosti: glej na grafiki
zaščitni sloji so sledeči: $c_{min} = 3,5 \text{ cm}$ priporočljiva 5cm
- Armatura: S 500-A (mrežna armatura)
 $f_{yk} = 50,0 \text{ kN/cm}^2$
 $E_s = 20000 \text{ kN/cm}^2$

S 500-B (rebrasta armatura, palice)
 $f_{yk} = 50,0 \text{ kN/cm}^2$
 $E_s = 20000 \text{ kN/cm}^2$
- Les: **C 24 - smreka, rezan les SIST EN 14081-1**
 $f_{m,k} = 2,4 \text{ kN/m}^2$
 $f_{t,0,k} = 1,4 \text{ kN/m}^2$
 $f_{c,0,k} = 2,1 \text{ kN/m}^2$
 $f_{v,k} = 0,25 \text{ kN/m}^2$
 $E_{0,mean} = 1100 \text{ kN/m}^2$
- Jeklo: **S 235-J0** $f_{yk} = 23,5 \text{ kN/cm}^2$
 $E_s = 21000 \text{ kN/cm}^2$
- Konstrukcijski vijaki: **8'8** SIST EN 14399
- Zvari: EN ISO 3834
- Kemijska sidra: navojna palica 8'8 + kemična masa (kot npr. Hilti HIT-HY 200-A)

Kvaliteta vseh osnovnih materialov in veznih sredstev je označena skladno s slovenskimi standardi SIST. Ne glede na način označevanja je izvajalec dolžan preveriti zahtevane kvalitete oz. predvsem zahtevane mehanske lastnosti materialov. Pri izdelavi projektne dokumentacije PZI za jekleno konstrukcijo morajo biti upoštevana predvsem določila iz standarda SIST EN 1993: Projektiranje jeklenih konstrukcij in SIST EN 1998: Projektiranje potresno odpornih konstrukcij. Specifikacije, lastnosti, proizvodnja in skladnost betona in jekla so določene s standardom. Jeklena konstrukcija je

2. NAČRT GRADBENIŠTA

izdelana iz konstrukcijskega jekla S 235 JR. Mehanske lastnosti mehkega konstrukcijskega jekla – meja plastičnosti in natezna trdnost – so določene s standardom SIST EN 10025. Izdelava in montaža nosilne jeklene konstrukcije mora biti v skladu s SIST EN 1090-1.

Pri zvarih mora biti meja plastičnosti, natezna trdnost, relativni porušni raztezek in žilavost dodatnega materiala vsaj enaka ali večja kot pri osnovnem materialu, ki ga varimo. V konstrukciji so predvideni kotni zvari, pri čemer je debelina zvara 0,46 (za S235) kratnik debeline najtanjšega materiala, ki se ga vari. Absolutno najmanjša debelina nosilnega zvara je 3,0 mm. Uporabljene so visokotrnostne vijačne zveze brez prednapetja, ki ustrezajo standardu SIST EN 15048.

Izbrani izvajalec mora izdelati načrt montaže jeklenih konstrukcij glede na uporabljeno opremo in mehanizacijo s katerim mora biti izkazana stabilnost konstrukcij v vseh fazah gradnje.

Tehnično poročilo, statično poročilo, načrt projekta za izvedbo, armaturni načrti, ... , je nujno treba gledati skupaj in upoštevati vsa navodila, določila in risbe, ki so vključene v teh dokumentih. V primeru ne jasnosti je treba kontaktirati pristojnega projektanta. Vse spremembe projekta potrjevati pisno.

2.2. ANALIZA VPLIVOV

2.2.1. STALNA TEŽA / LASTNA TEŽA

SIST EN 1991-1-1; 2

2.2.1. OBTEŽBA STREŠNE KONSTRUKCIJE

pozicija	ime sloja	debelina [m]	specifična tež [kN/m ³]	skupaj [kN/m ²]
1.	KRITINA/opečni zareznik/			0,45
2.	LETVE (5/4cm)			0,03
3.	KONTRA LETVE (8/5cm)			0,02
4.	SEKUNDRANA KRITINA (TAYVEK)			0,01
6.	NOSILNA LESENA KONSTRUKCIJA			program upošteva sam
7.	dodatno elektrama			
				0,35

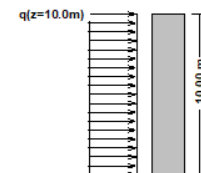
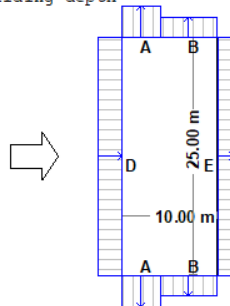
2.2.2. KORISTNA OBTEŽBA	SIST EN 1991-1-1; 2004				
Kategorije površine:	"C" - protori kjer se zbirajo ljudje	3,00	kN/m ²	kat:	C
	SKUPAJ $q =$ 3,00 kN/m²				
2.2.3. OBTEŽBA SNEGA	SIST EN 1991-1-3; 2004				
► klimatska cona :	A2				
► nadmorska višina :	536	m NM			
► oblika strehe :	DVOKAPNICA				
► naklon strehe :	25	°			
► snegolovi :	DA				
► Koeficijent izpostavljenosti C_e :	Običajen				
► Toplotni koeficijent C_t :	1				
• obtežba snega na tleh :	1,99	kN/m ²			
• oblikovni koeficijent strehe μ_1 :	0,8				
• obtežba snega na strehi :	1,60	kN/m ²			
2.2.4. OBTEŽBA VETRA	SIST EN 1991-1-4; 2004				
Območje: Velike Lašče	Cona:	1	proj.hitrost vetra	20	m/s

1. Veter fasada**WIND PRESSURE ON VERTICAL WALLS**

Eurocode 1 (EC1) Actions on structures, Wind actions , EN1991-1-4:2005

1.1. Rectangular plan building

Building height : $h = 10.00$ m
 Building cross wind dimension: $b = 25.00$ m
 Building depth : $d = 10.00$ m

1.2. Reference velocity

(EN1991-1-4, §4.2)

 $v_{bo} = 20.00$ m/s, , Zone: 1 $v_b = C_{dir} \cdot C_{season} \cdot V_{bo} = 20.00$ m/s1.3. Terrain effects

(EN1991-1-4, §4.3.2, Annex A)

Terrain category : III

(EN1991-1-4, Tab.4.1)

Area with regular cover of vegetation or buildings (villages, suburban terrain, forest)

Roughness factor $C_r(z)$

(EN1991-1-4, §4.3.2)

Terrain category: III, $z = 10.000$ m, $z_o = 0.300$ m, $z_{min} = 5$ m, $z_{max} = 200$ m, $z_{oII} = 0.050$ m $k_r = 0.19 \cdot (0.300/0.05)^{0.07} = 0.215$ $C_r(z) = k_r \cdot \ln(z/z_o) = 0.215 \times \ln(10.000/0.300) = 0.755$ Orography factor $C_o(z)$

(EN1991-1-4, §4.3.3)

 $C_o(z) = 1.000$

(EN1991-1-4, §4.3.3)

Turbulence factor K_t

(EN1991-1-4, §4.4)

 $K_t = 1.000$ Exposure factor $C_e(z)$

(EN1991-1-4, §4.5)

Terrain category: III

(EN1991-1-4, Tab.4.1)

 $z = 10.00$ m, $k_r = 0.215$, $I_v(z) = 0.285$, $C_e(z) = 1.709$

(EN1991-1-4, eq.A. 4.8, 4.7, 4.4, 4.3)

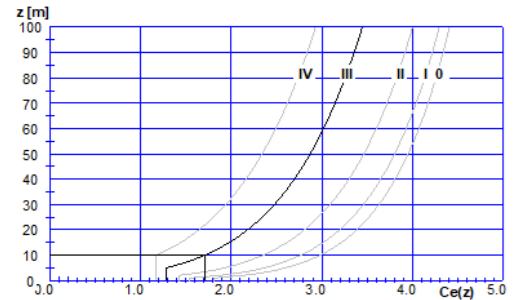
 $q(z) = C_e(z) \cdot (\frac{1}{2} \rho) \cdot V_b^2 = [0.001] \times 1.709 \times 0.625 \times 20.00^2 = 0.427$ kN/m²

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1.4. Wind peak velocity pressure $q(z)=C_e(z) \cdot q_b=C_e(z) \cdot (0.625) \cdot V_b^2$

(EN1991-1-4, §4.5)

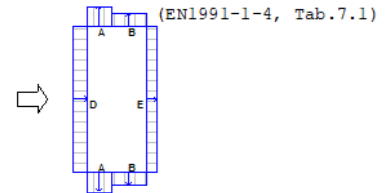
$V_b=20.00\text{m/sec}$
 $z=10.00\text{m}$, $C_r(z)=0.755$, $C_o(z)=1.000$, $K_t=1.000$
 $q(z)=C_e(z) \cdot (\frac{1}{2}\rho) \cdot V_b^2=[0.001]C_e(z) \times 0.625 \times 20.00^2 \text{ kN/m}^2$
 $c(z)=1.709$
 $q(z)=[10^{-3}] \times 1.709 \times 0.625 \times 20.00^2 = 0.43 \text{ kN/m}^2$

1.5. Wind forces on vertical walls

(EN1991-1-4, §7.2.2)

Wind pressure coefficients C_{pe}

$h/d=10.00/10.00=1.000$, $e=20.00\text{m}$
 Zone : A, (4.00xh), $C_{pe,10}=-1.20$, $C_{pe,1}=-1.40$
 Zone : B, (6.00xh), $C_{pe,10}=-0.80$, $C_{pe,1}=-1.10$
 Zone : D, (25.00xh), $C_{pe,10}=0.80$, $C_{pe,1}=1.00$
 Zone : E, (25.00xh), $C_{pe,10}=-0.50$, $C_{pe,1}=-0.50$

Wind pressure on wall surfaces $w_e=q(z) \cdot C_{pe}$ [kN/m²]

(EN1991-1-4, 5.1)

	A		B		D		E	
	$w_{e,10}$	$w_{e,1}$	$w_{e,10}$	$w_{e,1}$	$w_{e,10}$	$w_{e,1}$	$w_{e,10}$	$w_{e,1}$
$z=10.00 \sim 0.00\text{m}$,	-0.513	-0.598	-0.342	-0.470	0.342	0.427	-0.214	-0.214

Wind forces on wall surfaces $F_w=w_e \cdot A$ [kN]

			A (4.00m)	B (6.00m)	D (25.00m)	E (25.00m)
	z_c [m]	dz [m]	F_w [kN]	F_w [kN]	F_w [kN]	F_w [kN]
$z=10.00 \sim 0.00\text{m}$,	5.00	10.00	-20.510	-20.510	85.459	-53.412

Wind forces [kN] on wall surfaces, and moments [kNm] in respect to the base

Zone : A, Force $F_w = -20.51 \text{ kN}$, $z_c = 5.00\text{m}$, Moment $M_w = z_c \cdot F_w = 102.55 \text{ kNm}$
 Zone : B, Force $F_w = -20.51 \text{ kN}$, $z_c = 5.00\text{m}$, Moment $M_w = z_c \cdot F_w = 102.55 \text{ kNm}$
 Zone : D, Force $F_w = 85.46 \text{ kN}$, $z_c = 5.00\text{m}$, Moment $M_w = z_c \cdot F_w = 427.30 \text{ kNm}$
 Zone : E, Force $F_w = -53.41 \text{ kN}$, $z_c = 5.00\text{m}$, Moment $M_w = z_c \cdot F_w = 267.06 \text{ kNm}$

2. NAČRT GRADBENIŠTA

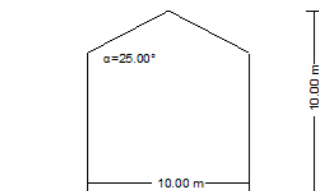
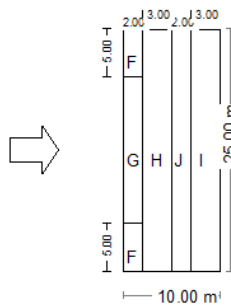
1. Veter streha

WIND PRESSURE ON ROOFS

Eurocode 1 (EC1) Actions on structures, Wind actions , EN1991-1-4:2005

1.1. Duopitch roof

Building height : $h = 10.00$ m
Building cross wind dimension: $b = 25.00$ m
Building depth : $d = 10.00$ m
Pitch angle : $\alpha_1 = 25.00^\circ$
Pitch angle : $\alpha_2 = 25.00^\circ$



1.2. Reference velocity

(EN1991-1-4, §4.2)

 $v_{bo} = 20.00$ m/s, , Zone: 1 $v_b = C_{dir} \cdot C_{season} \cdot V_{bo} = 20.00$ m/s

1.3. Terrain effects

(EN1991-1-4, §4.3.2, Annex A)

Terrain category : III

(EN1991-1-4, Tab.4.1)

Area with regular cover of vegetation or buildings (villages, suburban terrain, forest)

Roughness factor $C_r(z)$

(EN1991-1-4, §4.3.2)

Terrain category: III, $z = 10.000$ m, $z_0 = 0.300$ m, $z_{min} = 5$ m, $z_{max} = 200$ m, $z_{0II} = 0.050$ m $k_r = 0.19 \cdot (0.300/0.05)^{0.07} = 0.215$ $C_r(z) = k_r \cdot \ln(z/z_0) = 0.215 \times \ln(10.000/0.300) = 0.755$ Orography factor $C_o(z)$

(EN1991-1-4, §4.3.3)

 $C_o(z) = 1.000$

(EN1991-1-4, §4.3.3)

Turbulence factor K_t

(EN1991-1-4, §4.4)

 $K_t = 1.000$ Exposure factor $C_e(z)$

(EN1991-1-4, §4.5)

Terrain category: III

(EN1991-1-4, Tab.4.1)

 $z = 10.00$ m, $k_r = 0.215$, $I_v(z) = 0.285$, $C_e(z) = 1.709$

(EN1991-1-4, eq.A. 4.8, 4.7, 4.4, 4.3)

 $q(z) = C_e(z) \cdot \left(\frac{1}{2}\rho\right) \cdot V_b^2 = [0.001] \times 1.709 \times 0.625 \times 20.00^2 = 0.427$ kN/m²

2. NAČRT GRADBENIŠTA

1.4. Wind peak velocity pressure $q(z)=C_e(z) \cdot q_b=C_e(z) \cdot (0.625) \cdot V_b^2$

(EN1991-1-4, §4.5)

$$V_b=20.00\text{m/sec}$$

$$z=10.000\text{m}$$

$$C_r(z)=0.755$$

$$C_o(z)=1.000$$

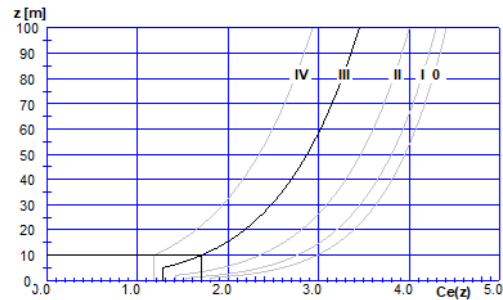
$$K_t=1.000$$

$$C_e(z)=1.709$$

$$q(z)=C_e(z) \cdot \left(\frac{1}{2}\rho\right) \cdot V_b^2$$

$$=[0.001] \times 1.709 \times 0.625 \times 20.00^2$$

$$= 0.427 \text{ kN/m}^2$$

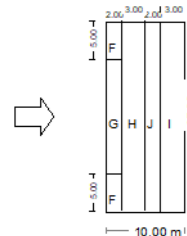


1.5. Wind forces on duopitch roof, wind direction: 0.00

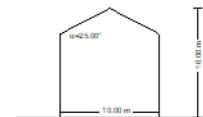
(EN1991-1-4, §7.2.5)

Wind pressure coefficients C_{pe} wind direction: $\theta=0.00$ $b=25.00\text{m}$, $d=10.00\text{m}$, $h=10.00\text{m}$, $e=\min(b, 2h)=20.00\text{m}$ $e/4=5.00\text{m}$, $e/10=2.00\text{m}$, $e/2=10.00\text{m}$ Pitch angle: $\alpha=25.000$

Zone : F, $A= 11.03\text{m}^2$, $C_{pe,10}=-0.63$, $C_{pe,1}=-1.67$
 Zone : F, $A= 11.03\text{m}^2$, $C_{pe,10}=+0.53$, $C_{pe,1}=+0.53$
 Zone : G, $A= 33.10\text{m}^2$, $C_{pe,10}=-0.60$, $C_{pe,1}=-1.50$
 Zone : G, $A= 33.10\text{m}^2$, $C_{pe,10}=+0.53$, $C_{pe,1}=+0.53$
 Zone : H, $A= 82.75\text{m}^2$, $C_{pe,10}=-0.23$, $C_{pe,1}=-0.23$
 Zone : H, $A= 82.75\text{m}^2$, $C_{pe,10}=+0.33$, $C_{pe,1}=+0.33$
 Zone : I, $A= 82.75\text{m}^2$, $C_{pe,10}=-0.40$, $C_{pe,1}=-0.40$
 Zone : I, $A= 82.75\text{m}^2$, $C_{pe,10}=+0.00$, $C_{pe,1}=+0.00$
 Zone : J, $A= 55.17\text{m}^2$, $C_{pe,10}=-0.67$, $C_{pe,1}=-0.83$
 Zone : J, $A= 55.17\text{m}^2$, $C_{pe,10}=+0.00$, $C_{pe,1}=+0.00$



(EN1991-1-4, Tab. 7.4a)

Wind pressure on roof surfaces $w_e=q(z) \cdot C_{pe}=0.427 \times C_{pe} \text{ [kN/m}^2\text{]}$

(EN1991-1-4, 5.1)

F		G		H		I		J	
$w_{e,10}$	$w_{e,1}$	$w_{e,10}$	$w_{e,1}$	$w_{e,10}$	$w_{e,1}$	$w_{e,10}$	$w_{e,1}$	$w_{e,10}$	$w_{e,1}$
-0.271	-0.712	-0.256	-0.641	-0.100	-0.100	-0.171	-0.171	-0.285	-0.356
+0.228	+0.228	+0.228	+0.228	+0.142	+0.142	+0.000	+0.000	+0.000	+0.000

Wind forces on roof surfaces $Q_e=w_{e,10} \cdot A \text{ [kN]}$

F		G		H		I		J	
Q_e	-2.986 kN	Q_e	-8.486 kN	Q_e	-8.251 kN	Q_e	-14.144 kN	Q_e	-15.716 kN
Q_e	+2.515 kN	Q_e	+7.544 kN	Q_e	+11.787 kN	Q_e	+0.000 kN	Q_e	+0.000 kN

Total force on roof surface (sum of all areas) : $F_l = -52.6 \text{ kN}$, $F = 24.4 \text{ kN}$ Total moment of roof force around left edge : $M_{yl} = -189.4 \text{ kNm}$, $M_y = 59.4 \text{ kNm}$ Total moment of roof force around top edge : $M_{xl} = -657.1 \text{ kNm}$, $M_x = 304.5 \text{ kNm}$

2. NAČRT GRADBENIŠTA

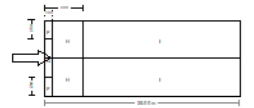
1.6. Wind forces on duopitch roof, wind direction: 90.00

(EN1991-1-4, §7.2.5)

Wind pressure coefficients C_{pe}

(EN1991-1-4, Tab. 7.4a)

wind direction: $\theta=90.00$
 $b=10.00\text{m}$, $d=25.00\text{m}$, $h=10.00\text{m}$, $e=\min(b,2h)=10.00\text{m}$
 $e/4=2.50\text{m}$, $e/10=1.00\text{m}$, $e/2=5.00\text{m}$
Pitch angle: $\alpha=25.000$



Zone : F, $A= 2.76\text{m}^2$, $C_{pe,10}=-1.17$, $C_{pe,1}=-1.67$
Zone : G, $A= 5.52\text{m}^2$, $C_{pe,10}=-1.37$, $C_{pe,1}=-2.00$
Zone : H, $A= 44.14\text{m}^2$, $C_{pe,10}=-0.73$, $C_{pe,1}=-1.20$
Zone : I, $A= 220.68\text{m}^2$, $C_{pe,10}=-0.50$, $C_{pe,1}=-0.50$

Wind pressure on roof surfaces $w_e=q(z) \cdot C_{pe}=0.427 \times C_{pe}$ [kN/m^2]

(EN1991-1-4, 5.1)

F		G		H		I	
$w_{e,10}$	$w_{e,1}$	$w_{e,10}$	$w_{e,1}$	$w_{e,10}$	$w_{e,1}$	$w_{e,10}$	$w_{e,1}$
-0.499	-0.712	-0.584	-0.855	-0.313	-0.513	-0.214	-0.214

Wind forces on roof surfaces $Q_e=w_{e,10} \cdot A$ [kN]

F		G		H		I	
Q_e	-1.375 kN	Q_e	-3.222 kN	Q_e	-13.830 kN	Q_e	-47.147 kN

Total force on roof surface (sum of all areas) : $F = -66.9$ kN
Total moment of roof force around left edge : $M_y = -327.7$ kNm
Total moment of roof force around top edge : $M_x = -744.8$ kNm

2.3. STATIČNA ANALIZA

2.3.1. ANALIZA OBJEKTA

Izračun krovnih slojev betona:

- Talna plošča

KROVNA PLAST BETONA			SIST EN 1992-1-1:2004			
IZBIRA IZPOSTAVLJENOSTI GLEDE NA POGOJE OKOLJA V SKLADU Z EN 206-1						XC1
RAZRED KONSTRUKCIJE						S4
PRIPOROČENA ŽIVLJENSKA DOBA				STAVBE		4
KROVNA SLOJ		$c_{nom} = c_{min} + \Delta c_{dev} =$		35	mm	
$c_{min} = \max \{c_{min,b}; c_{min,dur} + \Delta c_{dur,y} - \Delta c_{dur,st} - \Delta c_{dur,add}; 10\text{mm}\} =$						25 mm
$c_{min,b} = \emptyset =$	8	mm				
$c_{min,dur} =$	8	mm				
	25	mm				
$\Delta c_{dev} =$	10	mm				

- Ostale plošče/stene

KROVNA PLAST BETONA			SIST EN 1992-1-1:2004			
IZBIRA IZPOSTAVLJENOSTI GLEDE NA POGOJE OKOLJA V SKLADU Z EN 206-1						XC4
RAZRED KONSTRUKCIJE						S4
PRIPOROČENA ŽIVLJENSKA DOBA			STAVBE			4
KROVNA SLOJ		$c_{nom} = c_{min} + \Delta c_{dev} =$		25	mm	
$c_{min} = \max \{c_{min,b}; c_{min,dur} + \Delta c_{dur,y} - \Delta c_{dur,st} - \Delta c_{dur,add}; 10\text{mm}\} =$						15 mm
$c_{min,b} = \emptyset =$	8	mm				
$c_{min,dur} =$	8	mm				
	15	mm				
$\Delta c_{dev} =$	10	mm				

Za izračun glej prilogo v nadaljevanju.

Vhodni podatki - Konstrukcija

Shema nivojev

Naziv	z [m]	h [m]
	8.01	3.68
	4.33	1.18
	3.15	0.80

Naziv	z [m]	h [m]
	2.35	1.25
	1.10	1.10
	0.00	

Tabele materialov

No	Naziv materiala	E[kN/m ²]	μ	γ [kN/m ³]	α_t [1/C]	Em[kN/m ²]	μ_m
1	C 25/30	3.100e+7	0.20	25.00	1.000e-5	3.100e+7	0.20
2	Les-Iglavci-Masiven les	1.000e+7	0.20	5.00	1.000e-5	1.000e+7	0.20

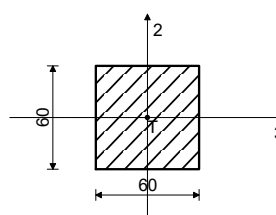
Seti plošč

No	d[m]	e[m]	Material	Tip preračuna	Ortotropija	E2[kN/m ²]	G[kN/m ²]	α
<1>	0.300	0.150	1	Tanka plošča	Izotropna			
<2>	0.200	0.100	1	Tanka plošča	Izotropna			
<3>	0.200	0.100	1	Tanka plošča	Izotropna			
<4>	0.200	0.100	1	Tanka plošča	Izotropna			
<5>	0.150	0.075	1	Tanka plošča	Izotropna			
<6>	0.200	0.100	1	Tanka plošča	Izotropna			

Seti gred

Set: 1 Prerez: b/d=60/60, Fiktivna ekscentričnost

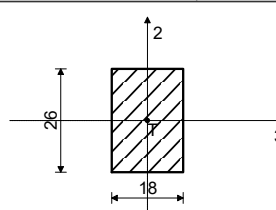
Mat.	A1	A2	A3	I1	I2	I3
1 - C 25/30	3.600e-1	3.000e-1	3.000e-1	1.825e-2	1.080e-2	1.080e-2



[cm]

Set: 2 Prerez: b/d=18/26, Fiktivna ekscentričnost

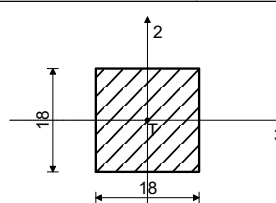
Mat.	A1	A2	A3	I1	I2	I3
2 - Les-Iglavci-M...	4.680e-2	3.900e-2	3.900e-2	2.892e-4	1.264e-4	2.636e-4



[cm]

Set: 3 Prerez: b/d=18/18, Fiktivna ekscentričnost

Mat.	A1	A2	A3	I1	I2	I3
2 - Les-Iglavci-M...	3.240e-2	2.700e-2	2.700e-2	1.478e-4	8.748e-5	8.748e-5



[cm]

Seti površinskih podpor

Set	K,R1	K,R2	K,R3
1	2.000e+4	2.000e+4	2.000e+4

Seti linijskih podpor

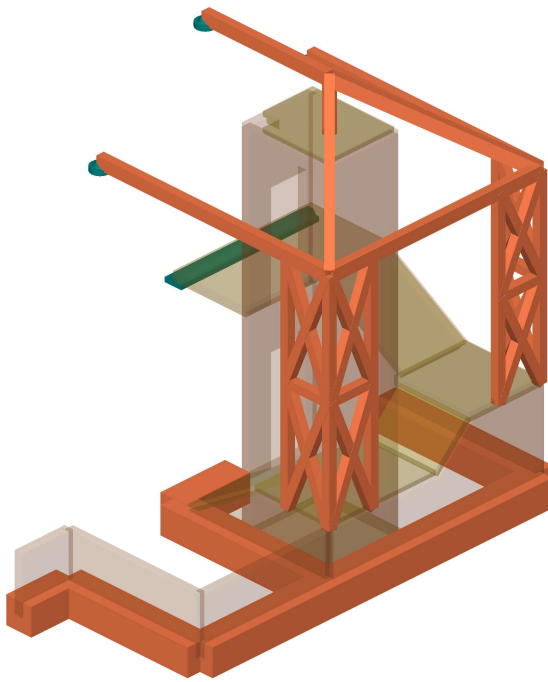
Set	K,R1	K,R2	K,R3	K,M1	Tla [m]
1	2.000e+4	2.000e+4	2.000e+4		0.600

Seti točkovnih podpor

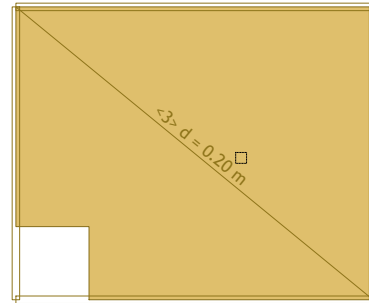
Set	K,R1	K,R2	K,R3	K,M1	K,M2	K,M3
1	1.000e+10	1.000e+10	1.000e+10			

Konture površinskih podpor

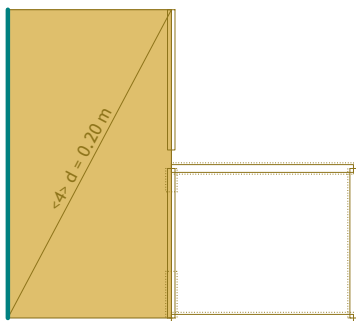
No	Konturna vozlišča	Sklop	Set
1	256-644-304-90-256	Nivo: [0.00 m]	1



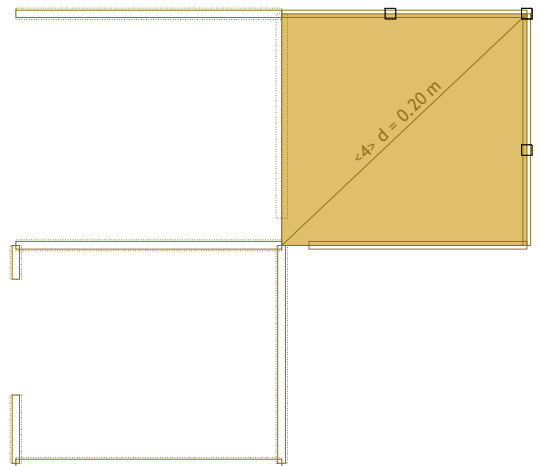
Izometrija



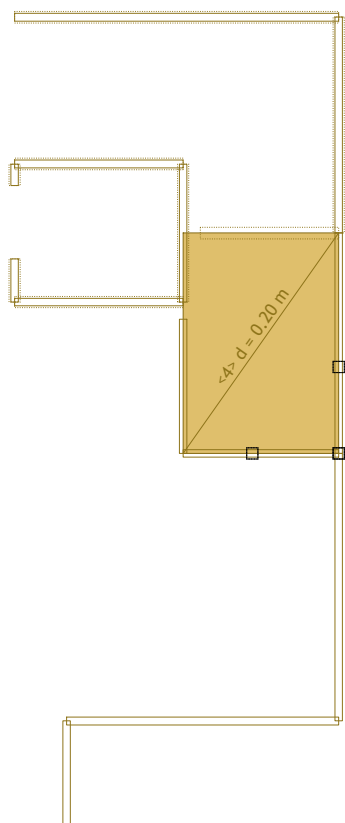
Nivo: [8.01 m]



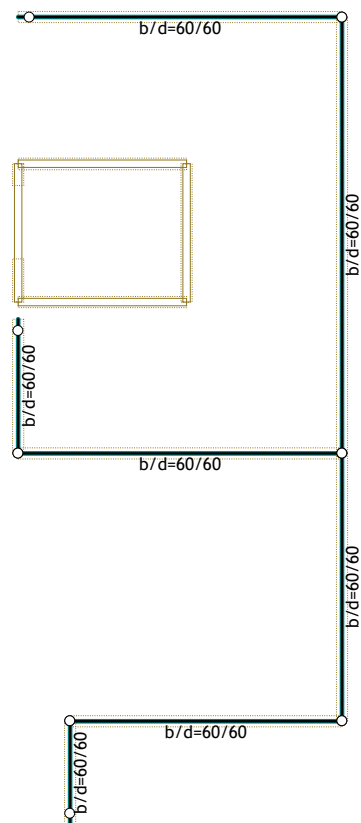
Nivo: [4.33 m]



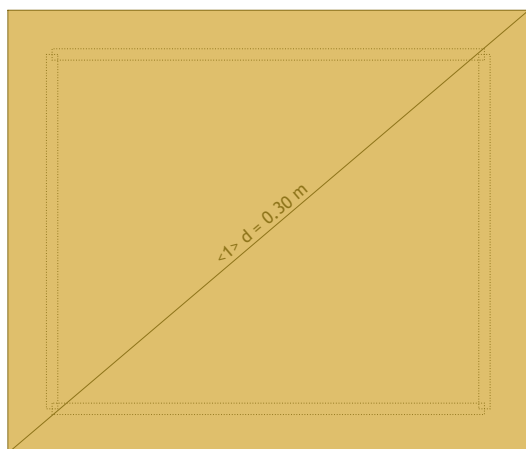
Nivo: [3.15 m]



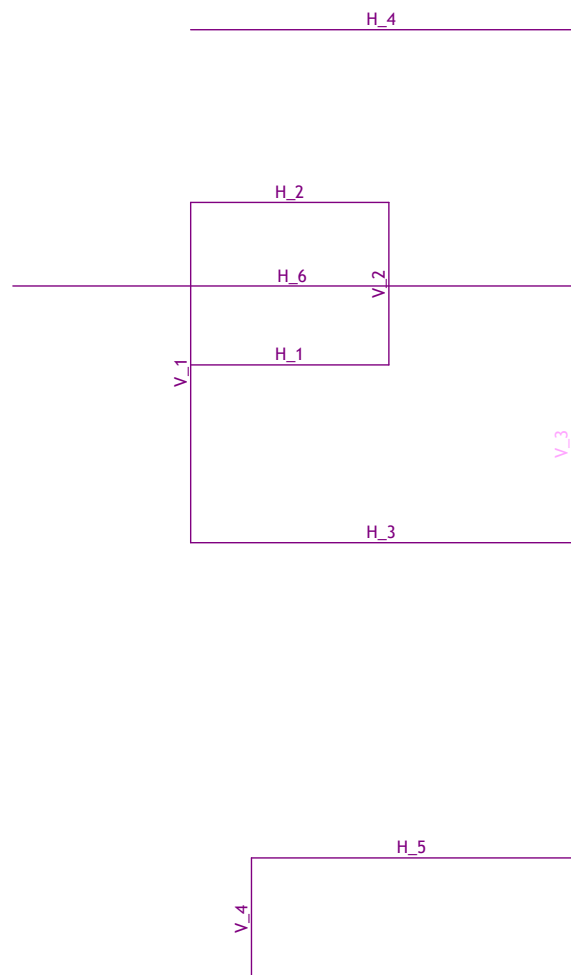
Nivo: [2.35 m]



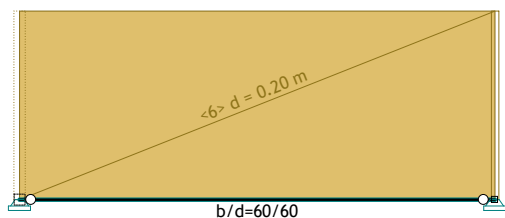
Nivo: [1.10 m]



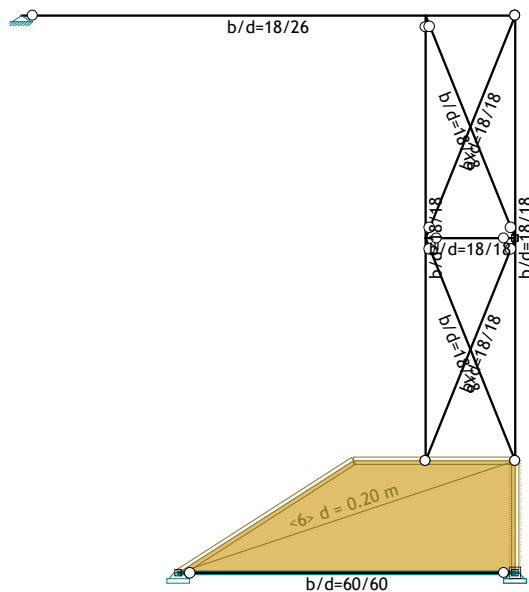
Nivo: [0.00 m]



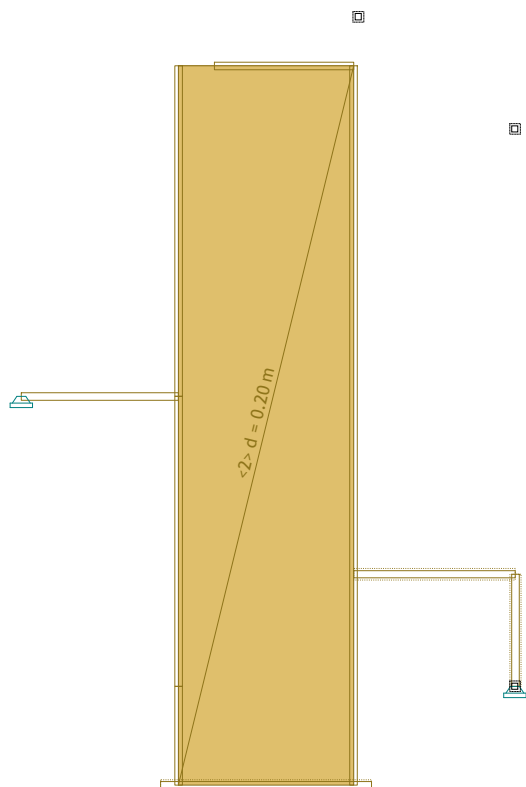
Dispozicija okvirjev



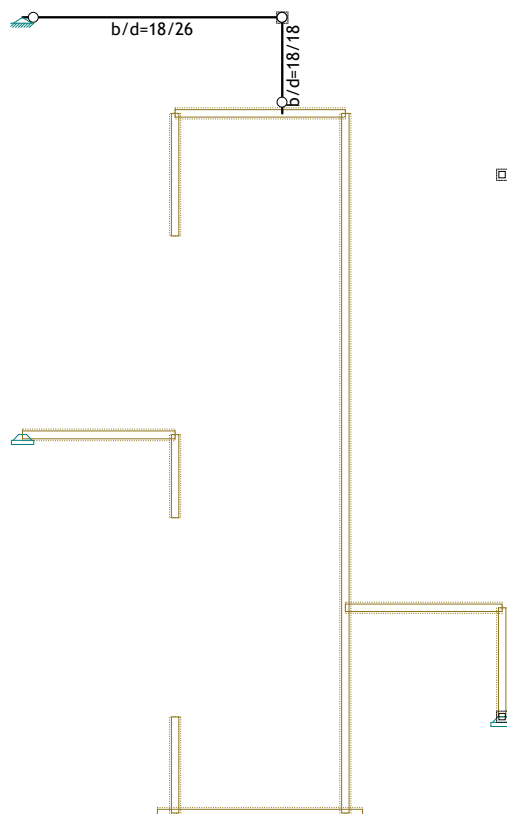
Okvir: H_5



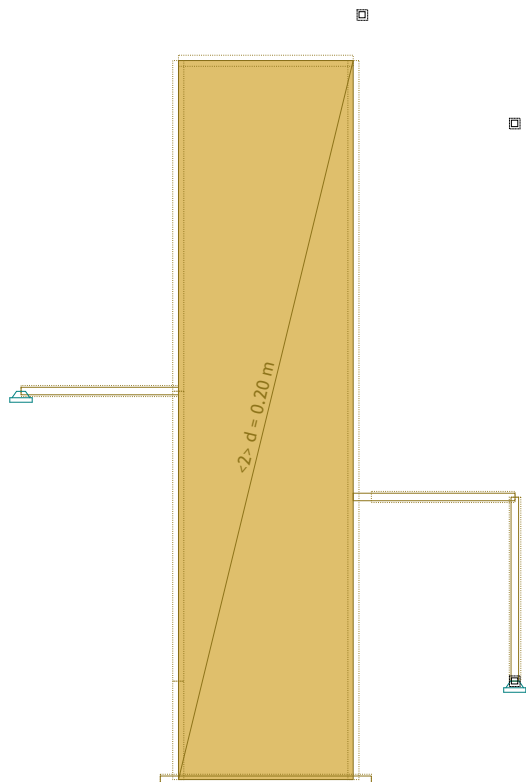
Okvir: H_3



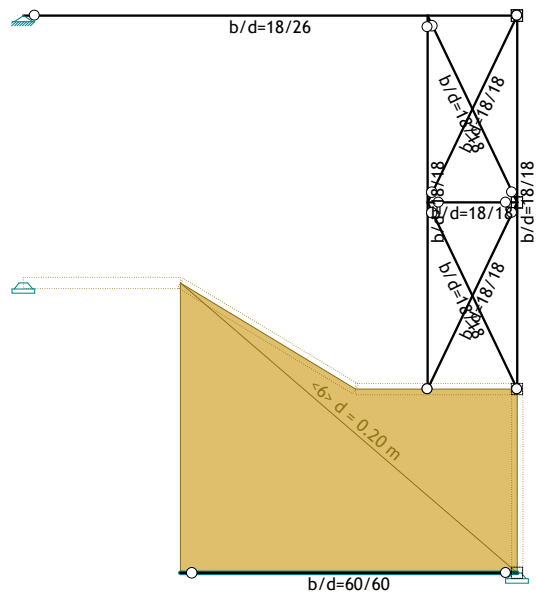
Okvir: H_1



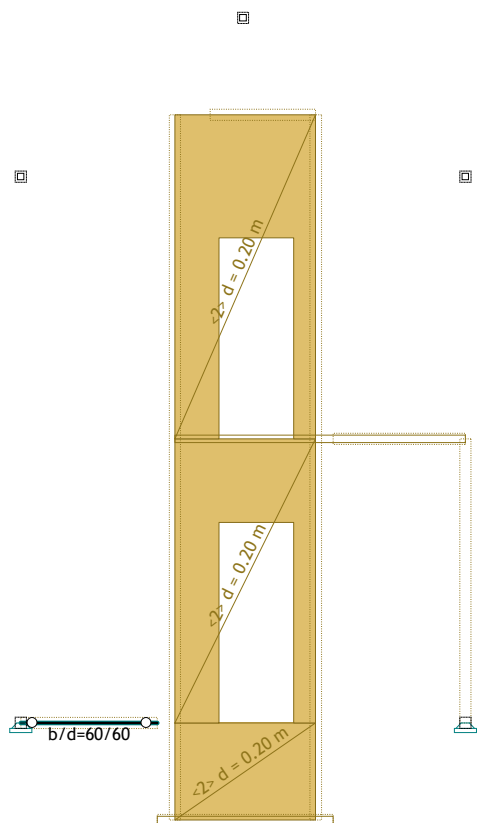
Okvir: H_6



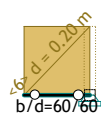
Okvir: H_2



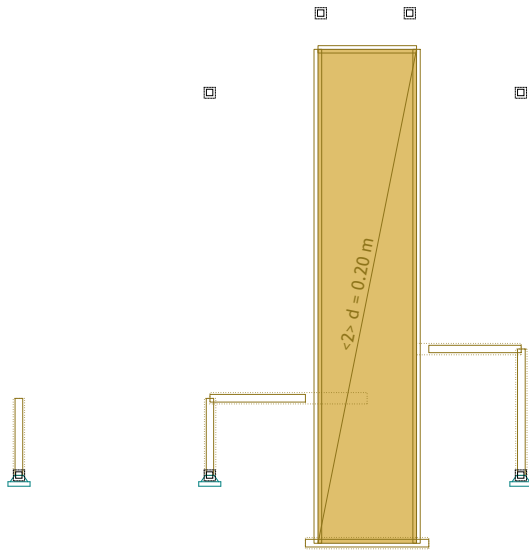
Okvir: H_4



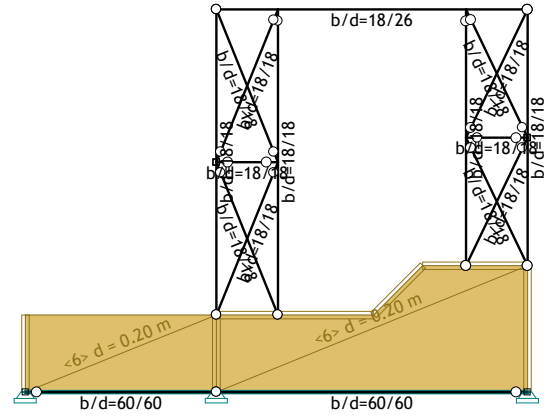
Okvir: V_1



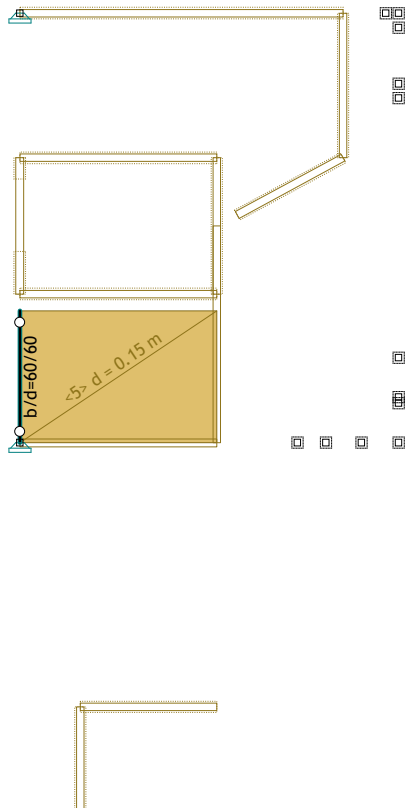
Okvir: V_4



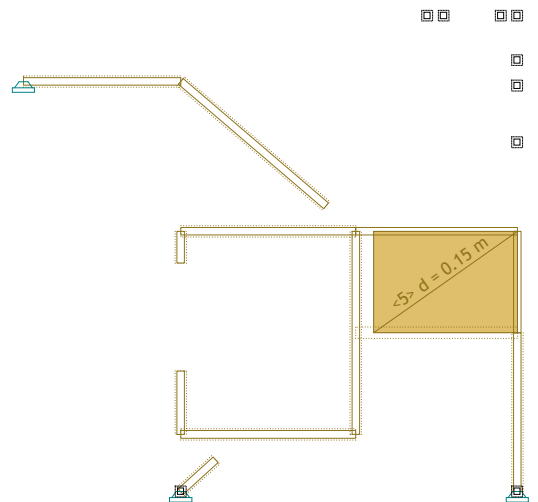
Okvir: V_2



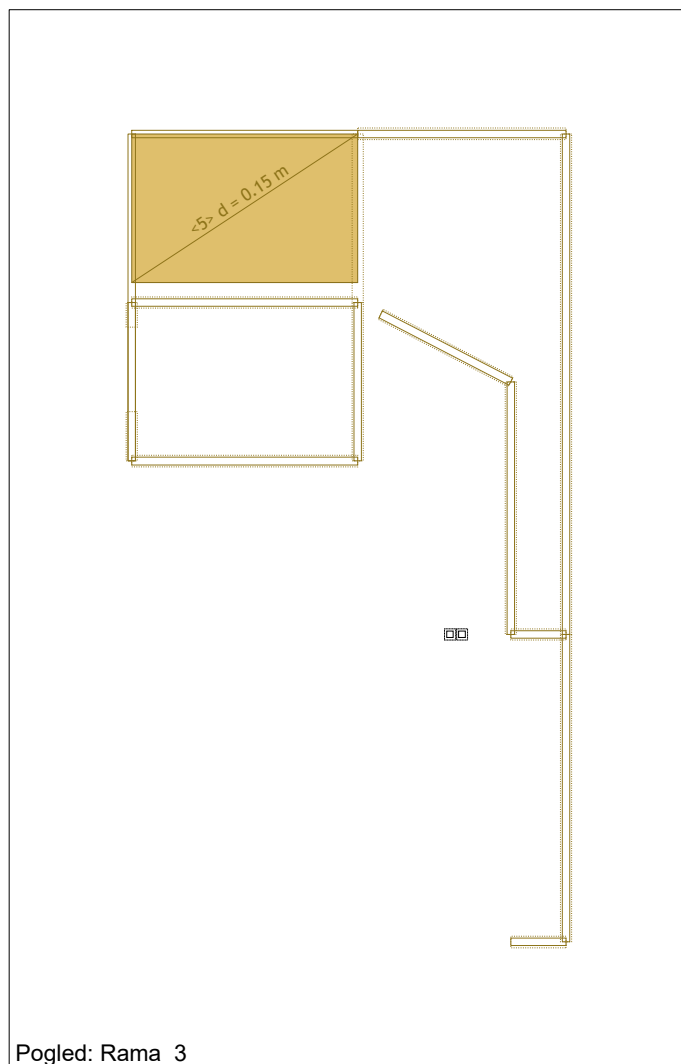
Okvir: V_3



Pogled: Rama_1



Pogled: Rama_2



Lista obtežnih primerov				
LC	Naziv	pX [kN]	pY [kN]	pZ [kN]
1	Lastna teža (g)	-0.00	0.00	-852.83
2	Lastna teža - pritisak zemljine	-297.49	103.16	0.00
3	Obtežba dvigalo	-2.50	-2.50	-10.00
4	Koristna teža	0.00	0.00	-67.92
5	Sneg	0.00	0.00	-44.30
6	Veter x	-10.00	0.00	0.00
7	Veter y	0.00	5.00	0.00
8	Potres x (+e)			
9	Potres x (-e)			
10	Potres y (+e)			
11	Potres y (-e)			
12	Komb.: 1.35xI+1.35xII+1.05xIII+ +1.05xIV+1.5xV+0.9xVII	-404.24	141.14	-1299.58
13	Komb.: 1.35xI+1.35xII+1.05xIII+ +1.05xIV+1.5xV+0.9xVI	-413.24	136.64	-1299.58
14	Komb.: 1.35xI+1.35xII+1.05xIII+ +1.05xIV+0.75xV+1.5xVII	-404.24	144.14	-1266.35
15	Komb.: 1.35xI+1.35xII+1.05xIII+ +1.05xIV+0.75xV+1.5xVI	-419.24	136.64	-1266.35
16	Komb.: 1.35xI+1.35xII+1.05xIII+1.5xIV+ +0.75xV+0.9xVII	-404.24	141.14	-1296.92
17	Komb.: 1.35xI+1.35xII+1.05xIII+ +1.5xIV+0.75xV+0.9xVI	-413.24	136.64	-1296.92
18	Komb.: 1.35xI+1.35xII+1.5xIII+1.05xIV+ +0.75xV+0.9xVII	-405.37	140.02	-1270.85
19	Komb.: 1.35xI+1.35xII+1.5xIII+ +1.05xIV+0.75xV+0.9xVI	-414.37	135.52	-1270.85
20	Komb.: I+1.35xII+1.05xIII+1.05xIV+ +1.5xV+0.9xVII	-404.24	141.14	-1001.09
21	Komb.: I+1.35xII+1.05xIII+1.05xIV+ +1.5xV+0.9xVI	-413.24	136.64	-1001.09
22	Komb.: 1.35xI+II+1.05xIII+1.05xIV+ +1.5xV+0.9xVII	-300.12	105.04	-1299.58
23	Komb.: 1.35xI+II+1.05xIII+1.05xIV+ +1.5xV+0.9xVI	-309.12	100.54	-1299.58
24	Komb.: I+1.35xII+1.05xIII+1.05xIV+ +0.75xV+1.5xVII	-404.24	144.14	-967.86
25	Komb.: I+1.35xII+1.05xIII+1.05xIV+ +0.75xV+1.5xVI	-419.24	136.64	-967.86
26	Komb.: 1.35xI+II+1.05xIII+1.05xIV+ +0.75xV+1.5xVII	-300.12	108.04	-1266.35
27	Komb.: 1.35xI+II+1.05xIII+1.05xIV+ +0.75xV+1.5xVI	-315.12	100.54	-1266.35
28	Komb.: I+1.35xII+1.05xIII+1.5xIV+ +0.75xV+0.9xVII	-404.24	141.14	-998.43
29	Komb.: I+1.35xII+1.05xIII+1.5xIV+ +0.75xV+0.9xVI	-413.24	136.64	-998.43
30	Komb.: I+1.35xII+1.5xIII+1.05xIV+ +0.75xV+0.9xVII	-405.37	140.02	-972.36
31	Komb.: I+1.35xII+1.5xIII+1.05xIV+ +0.75xV+0.9xVI	-414.37	135.52	-972.36
32	Komb.: 1.35xI+II+1.05xIII+1.5xIV+ +0.75xV+0.9xVII	-300.12	105.04	-1296.92
33	Komb.: 1.35xI+II+1.05xIII+1.5xIV+ +0.75xV+0.9xVI	-309.12	100.54	-1296.92
34	Komb.: 1.35xI+II+1.5xIII+1.05xIV+ +0.75xV+0.9xVII	-301.24	103.91	-1270.85
35	Komb.: 1.35xI+II+1.5xIII+1.05xIV+ +0.75xV+0.9xVI	-310.24	99.41	-1270.85
36	Komb.: I+II+1.05xIII+1.05xIV+1.5xV+ +0.9xVII	-300.12	105.04	-1001.09
37	Komb.: I+II+1.05xIII+1.05xIV+1.5xV+ +0.9xVI	-309.12	100.54	-1001.09
38	Komb.: I+II+1.05xIII+1.05xIV+0.75xV+ +1.5xVII	-300.12	108.04	-967.86
39	Komb.: I+II+1.05xIII+1.05xIV+0.75xV+ +1.5xVI	-315.12	100.54	-967.86
40	Komb.: 1.35xI+1.35xII+1.05xIII+ +1.05xIV+1.5xVII	-404.24	144.14	-1233.13
41	Komb.: 1.35xI+1.35xII+1.05xIII+ +1.05xIV+1.5xVI	-419.24	136.64	-1233.13
42	Komb.: 1.35xI+1.35xII+1.05xIII+ +1.05xIV+1.5xV	-404.24	136.64	-1299.58
43	Komb.: I+II+1.05xIII+1.5xIV+0.75xV+ +0.9xVII	-300.12	105.04	-998.43
44	Komb.: I+II+1.05xIII+1.5xIV+0.75xV+ +0.9xVI	-309.12	100.54	-998.43
45	Komb.: I+II+1.5xIII+1.05xIV+0.75xV+ +0.9xVII	-301.24	103.91	-972.36
46	Komb.: I+II+1.5xIII+1.05xIV+0.75xV+ +0.9xVI	-310.24	99.41	-972.36
47	Komb.: 1.35xI+1.35xII+1.05xIV+1.5xV+ +0.9xVII	-401.62	143.77	-1289.08
48	Komb.: 1.35xI+1.35xII+1.05xIV+1.5xV+ +0.9xVI	-410.62	139.27	-1289.08
49	Komb.: 1.35xI+1.35xII+1.05xIII+1.5xV+ +0.9xVII	-404.24	141.14	-1228.26
50	Komb.: 1.35xI+1.35xII+1.05xIII+1.5xV+ +0.9xVI	-413.24	136.64	-1228.26
51	Komb.: 1.35xI+1.35xII+1.05xIII+1.5xIV+ +0.9xVII	-404.24	141.14	-1263.69
52	Komb.: 1.35xI+1.35xII+1.05xIII+ +1.5xIV+0.9xVI	-413.24	136.64	-1263.69
53	Komb.: 1.35xI+1.35xII+1.5xIII+1.05xIV+ +0.9xVII	-405.37	140.02	-1237.63
54	Komb.: 1.35xI+1.35xII+1.5xIII+ +1.05xIV+0.9xVI	-414.37	135.52	-1237.63
55	Komb.: 1.35xI+1.35xII+1.05xIV+ +0.75xV+1.5xVII	-401.62	146.77	-1255.85
56	Komb.: 1.35xI+1.35xII+1.05xIV+	-416.62	139.27	-1255.85

Lista obtežnih primerov				
LC	Naziv	pX [kN]	pY [kN]	pZ [kN]
	+0.75xV+1.5xVI			
57	Komb.: 1.35xI+1.35xII+1.05xIII+ +0.75xV+1.5xVII	-404.24	144.14	-1195.04
58	Komb.: 1.35xI+1.35xII+1.05xIII+ +0.75xV+1.5xVI	-419.24	136.64	-1195.04
59	Komb.: 1.35xI+1.35xII+1.05xIII+ +1.5xIV+0.75xV	-404.24	136.64	-1296.92
60	Komb.: 1.35xI+1.35xII+1.5xIII+ +1.05xIV+0.75xV	-405.37	135.52	-1270.85
61	Komb.: I+1.35xII+1.05xIII+1.05xIV+ +1.5xVII	-404.24	144.14	-934.64
62	Komb.: I+1.35xII+1.05xIII+1.05xIV+ +1.5xVI	-419.24	136.64	-934.64
63	Komb.: I+1.35xII+1.05xIII+1.05xIV+ +1.5xV	-404.24	136.64	-1001.09
64	Komb.: 1.35xI+II+1.05xIII+1.05xIV+ +1.5xVII	-300.12	108.04	-1233.13
65	Komb.: 1.35xI+II+1.05xIII+1.05xIV+ +1.5xVI	-315.12	100.54	-1233.13
66	Komb.: 1.35xI+II+1.05xIII+1.05xIV+ +1.5xV	-300.12	100.54	-1299.58
67	Komb.: 1.35xI+1.35xII+1.5xIV+0.75xV+ +0.9xVII	-401.62	143.77	-1286.42
68	Komb.: 1.35xI+1.35xII+1.5xIV+0.75xV+ +0.9xVI	-410.62	139.27	-1286.42
69	Komb.: 1.35xI+1.35xII+1.5xIII+0.75xV+ +0.9xVII	-405.37	140.02	-1199.54
70	Komb.: 1.35xI+1.35xII+1.5xIII+0.75xV+ +0.9xVI	-414.37	135.52	-1199.54
71	Komb.: I+1.35xII+1.05xIV+1.5xV+ +0.9xVII	-401.62	143.77	-990.59
72	Komb.: I+1.35xII+1.05xIV+1.5xV+ +0.9xVI	-410.62	139.27	-990.59
73	Komb.: I+1.35xII+1.05xIII+1.5xV+ +0.9xVII	-404.24	141.14	-929.77
74	Komb.: I+1.35xII+1.05xIII+1.5xV+ +0.9xVI	-413.24	136.64	-929.77
75	Komb.: I+1.35xII+1.05xIII+1.5xIV+ +0.9xVII	-404.24	141.14	-965.20
76	Komb.: I+1.35xII+1.05xIII+1.5xIV+ +0.9xVI	-413.24	136.64	-965.20
77	Komb.: I+1.35xII+1.5xIII+1.05xIV+ +0.9xVII	-405.37	140.02	-939.14
78	Komb.: I+1.35xII+1.5xIII+1.05xIV+ +0.9xVI	-414.37	135.52	-939.14
79	Komb.: 1.35xI+II+1.05xIV+1.5xV+ +0.9xVII	-297.49	107.66	-1289.08
80	Komb.: 1.35xI+II+1.05xIV+1.5xV+ +0.9xVI	-306.49	103.16	-1289.08
81	Komb.: 1.35xI+II+1.05xIII+1.5xV+ +0.9xVII	-300.12	105.04	-1228.26
82	Komb.: 1.35xI+II+1.05xIII+1.5xV+ +0.9xVI	-309.12	100.54	-1228.26
83	Komb.: 1.35xI+II+1.05xIII+1.5xIV+ +0.9xVII	-300.12	105.04	-1263.69
84	Komb.: 1.35xI+II+1.05xIII+1.5xIV+ +0.9xVI	-309.12	100.54	-1263.69
85	Komb.: 1.35xI+II+1.5xIII+1.05xIV+ +0.9xVII	-301.24	103.91	-1237.63
86	Komb.: 1.35xI+II+1.5xIII+1.05xIV+ +0.9xVI	-310.24	99.41	-1237.63
87	Komb.: I+1.35xII+1.05xIV+0.75xV+ +1.5xVII	-401.62	146.77	-957.36
88	Komb.: I+1.35xII+1.05xIV+0.75xV+ +1.5xVI	-416.62	139.27	-957.36
89	Komb.: I+1.35xII+1.05xIII+0.75xV+ +1.5xVII	-404.24	144.14	-896.55
90	Komb.: I+1.35xII+1.05xIII+0.75xV+ +1.5xVI	-419.24	136.64	-896.55
91	Komb.: I+1.35xII+1.05xIII+1.5xIV+ +0.75xV	-404.24	136.64	-998.43
92	Komb.: I+1.35xII+1.5xIII+1.05xIV+ +0.75xV	-405.37	135.52	-972.36
93	Komb.: 1.35xI+II+1.05xIV+0.75xV+ +1.5xVII	-297.49	110.66	-1255.85
94	Komb.: 1.35xI+II+1.05xIV+0.75xV+ +1.5xVI	-312.49	103.16	-1255.85
95	Komb.: 1.35xI+II+1.05xIII+0.75xV+ +1.5xVII	-300.12	108.04	-1195.04
96	Komb.: 1.35xI+II+1.05xIII+0.75xV+ +1.5xVI	-315.12	100.54	-1195.04
97	Komb.: 1.35xI+II+1.05xIII+1.5xIV+ +0.75xV	-300.12	100.54	-1296.92
98	Komb.: 1.35xI+II+1.5xIII+1.05xIV+ +0.75xV	-301.24	99.41	-1270.85
99	Komb.: I+II+1.05xIII+1.05xIV+1.5xVII	-300.12	108.04	-934.64
100	Komb.: I+II+1.05xIII+1.05xIV+1.5xVI	-315.12	100.54	-934.64
101	Komb.: I+II+1.05xIII+1.05xIV+1.5xV	-300.12	100.54	-1001.09
102	Komb.: I+1.35xII+1.5xIV+0.75xV+ +0.9xVII	-401.62	143.77	-987.93
103	Komb.: I+1.35xII+1.5xIV+0.75xV+ +0.9xVI	-410.62	139.27	-987.93
104	Komb.: I+1.35xII+1.5xIII+0.75xV+ +0.9xVII	-405.37	140.02	-901.05
105	Komb.: I+1.35xII+1.5xIII+0.75xV+ +0.9xVI	-414.37	135.52	-901.05
106	Komb.: 1.35xI+II+1.5xIV+0.75xV+ +0.9xVII	-297.49	107.66	-1286.42
107	Komb.: 1.35xI+II+1.5xIV+0.75xV+ +0.9xVI	-306.49	103.16	-1286.42

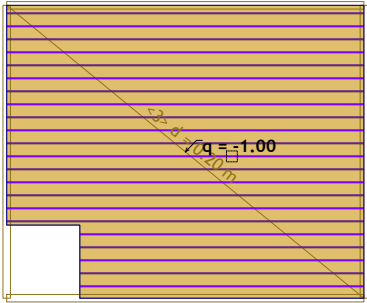
Lista obtežnih primerov				
LC	Naziv	pX [kN]	pY [kN]	pZ [kN]
108	Komb.: 1.35xI+II+1.5xIII+0.75xV+ +0.9xVII	-301.24	103.91	-1199.54
109	Komb.: 1.35xI+II+1.5xIII+0.75xV+ +0.9xVI	-310.24	99.41	-1199.54
110	Komb.: I+II+1.05xIV+1.5xV+0.9xVII	-297.49	107.66	-990.59
111	Komb.: I+II+1.05xIV+1.5xV+0.9xVI	-306.49	103.16	-990.59
112	Komb.: I+II+1.05xIII+1.5xV+0.9xVII	-300.12	105.04	-929.77
113	Komb.: I+II+1.05xIII+1.5xV+0.9xVI	-309.12	100.54	-929.77
114	Komb.: I+II+1.05xIII+1.5xIV+0.9xVII	-300.12	105.04	-965.20
115	Komb.: I+II+1.05xIII+1.5xIV+0.9xVI	-309.12	100.54	-965.20
116	Komb.: I+II+1.5xIII+1.05xIV+0.9xVII	-301.24	103.91	-939.14
117	Komb.: I+II+1.5xIII+1.05xIV+0.9xVI	-310.24	99.41	-939.14
118	Komb.: I+II+1.05xIV+0.75xV+1.5xVII	-297.49	110.66	-957.36
119	Komb.: I+II+1.05xIV+0.75xV+1.5xVI	-312.49	103.16	-957.36
120	Komb.: I+II+1.05xIII+0.75xV+1.5xVII	-300.12	108.04	-896.55
121	Komb.: I+II+1.05xIII+0.75xV+1.5xVI	-315.12	100.54	-896.55
122	Komb.: I+II+1.05xIII+1.5xIV+0.75xV	-300.12	100.54	-998.43
123	Komb.: I+II+1.5xIII+1.05xIV+0.75xV	-301.24	99.41	-972.36
124	Komb.: 1.35xI+1.35xII+1.05xIV+ +1.5xVII	-401.62	146.77	-1222.63
125	Komb.: 1.35xI+1.35xII+1.05xIV+1.5xVI	-416.62	139.27	-1222.63
126	Komb.: 1.35xI+1.35xII+1.05xIV+1.5xV	-401.62	139.27	-1289.08
127	Komb.: 1.35xI+1.35xII+1.05xIII+ +1.5xVII	-404.24	144.14	-1161.82
128	Komb.: 1.35xI+1.35xII+1.05xIII+1.5xVI	-419.24	136.64	-1161.82
129	Komb.: 1.35xI+1.35xII+1.05xIII+1.5xV	-404.24	136.64	-1228.26
130	Komb.: 1.35xI+1.35xII+1.05xIII+1.5xIV	-404.24	136.64	-1263.69
131	Komb.: 1.35xI+1.35xII+1.5xIII+1.05xIV	-405.37	135.52	-1237.63
132	Komb.: I+II+1.5xIV+0.75xV+0.9xVII	-297.49	107.66	-987.93
133	Komb.: I+II+1.5xIV+0.75xV+0.9xVI	-306.49	103.16	-987.93
134	Komb.: I+II+1.5xIII+0.75xV+0.9xVII	-301.24	103.91	-901.05
135	Komb.: I+II+1.5xIII+0.75xV+0.9xVI	-310.24	99.41	-901.05
136	Komb.: 1.35xI+1.35xII+1.5xV+0.9xVII	-401.62	143.77	-1217.76
137	Komb.: 1.35xI+1.35xII+1.5xV+0.9xVI	-410.62	139.27	-1217.76
138	Komb.: 1.35xI+1.35xII+1.5xIV+0.9xVII	-401.62	143.77	-1253.19
139	Komb.: 1.35xI+1.35xII+1.5xIV+0.9xVI	-410.62	139.27	-1253.19
140	Komb.: 1.35xI+1.35xII+1.5xIII+0.9xVII	-405.37	140.02	-1166.32
141	Komb.: 1.35xI+1.35xII+1.5xIII+0.9xVI	-414.37	135.52	-1166.32
142	Komb.: 1.35xI+1.35xII+0.75xV+1.5xVII	-401.62	146.77	-1184.54
143	Komb.: 1.35xI+1.35xII+0.75xV+1.5xVI	-416.62	139.27	-1184.54
144	Komb.: 1.35xI+1.35xII+1.5xIV+0.75xV	-401.62	139.27	-1286.42
145	Komb.: 1.35xI+1.35xII+1.5xIII+0.75xV	-405.37	135.52	-1199.54
146	Komb.: I+1.35xII+1.05xIV+1.5xVII	-401.62	146.77	-924.14
147	Komb.: I+1.35xII+1.05xIV+1.5xVI	-416.62	139.27	-924.14
148	Komb.: I+1.35xII+1.05xIV+1.5xV	-401.62	139.27	-990.59
149	Komb.: I+1.35xII+1.05xIII+1.5xVII	-404.24	144.14	-863.33
150	Komb.: I+1.35xII+1.05xIII+1.5xVI	-419.24	136.64	-863.33
151	Komb.: I+1.35xII+1.05xIII+1.5xV	-404.24	136.64	-929.77
152	Komb.: I+1.35xII+1.05xIII+1.5xIV	-404.24	136.64	-965.20
153	Komb.: I+1.35xII+1.5xIII+1.05xIV	-405.37	135.52	-939.14
154	Komb.: 1.35xI+II+1.05xIV+1.5xVII	-297.49	110.66	-1222.63
155	Komb.: 1.35xI+II+1.05xIV+1.5xVI	-312.49	103.16	-1222.63
156	Komb.: 1.35xI+II+1.05xIV+1.5xV	-297.49	103.16	-1289.08
157	Komb.: 1.35xI+II+1.05xIII+1.5xVII	-300.12	108.04	-1161.82
158	Komb.: 1.35xI+II+1.05xIII+1.5xVI	-315.12	100.54	-1161.82
159	Komb.: 1.35xI+II+1.05xIII+1.5xV	-300.12	100.54	-1228.26
160	Komb.: 1.35xI+II+1.05xIII+1.5xIV	-300.12	100.54	-1263.69
161	Komb.: 1.35xI+II+1.5xIII+1.05xIV	-301.24	99.41	-1237.63
162	Komb.: I+1.35xII+1.5xV+0.9xVII	-401.62	143.77	-919.27
163	Komb.: I+1.35xII+1.5xV+0.9xVI	-410.62	139.27	-919.27
164	Komb.: I+1.35xII+1.5xIV+0.9xVII	-401.62	143.77	-954.70
165	Komb.: I+1.35xII+1.5xIV+0.9xVI	-410.62	139.27	-954.70
166	Komb.: I+1.35xII+1.5xIII+0.9xVII	-405.37	140.02	-867.83
167	Komb.: I+1.35xII+1.5xIII+0.9xVI	-414.37	135.52	-867.83
168	Komb.: 1.35xI+II+1.5xV+0.9xVII	-297.49	107.66	-1217.76
169	Komb.: 1.35xI+II+1.5xV+0.9xVI	-306.49	103.16	-1217.76
170	Komb.: 1.35xI+II+1.5xIV+0.9xVII	-297.49	107.66	-1253.19
171	Komb.: 1.35xI+II+1.5xIV+0.9xVI	-306.49	103.16	-1253.19
172	Komb.: 1.35xI+II+1.5xIII+0.9xVII	-301.24	103.91	-1166.32
173	Komb.: 1.35xI+II+1.5xIII+0.9xVI	-310.24	99.41	-1166.32
174	Komb.: I+1.35xII+0.75xV+1.5xVII	-401.62	146.77	-886.05
175	Komb.: I+1.35xII+0.75xV+1.5xVI	-416.62	139.27	-886.05
176	Komb.: I+1.35xII+1.5xIV+0.75xV	-401.62	139.27	-987.93
177	Komb.: I+1.35xII+1.5xIII+0.75xV	-405.37	135.52	-901.05
178	Komb.: 1.35xI+II+0.75xV+1.5xVII	-297.49	110.66	-1184.54
179	Komb.: 1.35xI+II+0.75xV+1.5xVI	-312.49	103.16	-1184.54
180	Komb.: 1.35xI+II+1.5xIV+0.75xV	-297.49	103.16	-1286.42
181	Komb.: 1.35xI+II+1.5xIII+0.75xV	-301.24	99.41	-1199.54
182	Komb.: I+II+1.05xIV+1.5xVII	-297.49	110.66	-924.14
183	Komb.: I+II+1.05xIV+1.5xVI	-312.49	103.16	-924.14
184	Komb.: I+II+1.05xIV+1.5xV	-297.49	103.16	-990.59
185	Komb.: I+II+1.05xIII+1.5xVII	-300.12	108.04	-863.33
186	Komb.: I+II+1.05xIII+1.5xVI	-315.12	100.54	-863.33
187	Komb.: I+II+1.05xIII+1.5xV	-300.12	100.54	-929.77
188	Komb.: I+II+1.05xIII+1.5xIV	-300.12	100.54	-965.20
189	Komb.: I+II+1.5xIII+1.05xIV	-301.24	99.41	-939.14
190	Komb.: I+II+1.5xV+0.9xVII	-297.49	107.66	-919.27
191	Komb.: I+II+1.5xV+0.9xVI	-306.49	103.16	-919.27
192	Komb.: I+II+1.5xIV+0.9xVII	-297.49	107.66	-954.70
193	Komb.: I+II+1.5xIV+0.9xVI	-306.49	103.16	-954.70
194	Komb.: I+II+1.5xIII+0.9xVII	-301.24	103.91	-867.83
195	Komb.: I+II+1.5xIII+0.9xVI	-310.24	99.41	-867.83
196	Komb.: I+II+0.75xV+1.5xVII	-297.49	110.66	-886.05
197	Komb.: I+II+0.75xV+1.5xVI	-312.49	103.16	-886.05
198	Komb.: I+II+1.5xIV+0.75xV	-297.49	103.16	-987.93
199	Komb.: I+II+1.5xIII+0.75xV	-301.24	99.41	-901.05
200	Komb.: I+II+0.6xIII+0.6xIV-1xVII			
201	Komb.: I+II+0.6xIII+0.6xIV-1xIX			
202	Komb.: I+II+0.6xIII+0.6xIV-1xX			
203	Komb.: I+II+0.6xIII+0.6xIV-1xXI			

Lista obtežnih primerov				
LC	Naziv	pX [kN]	pY [kN]	pZ [kN]
204	Komb.: I+II+0.6xIII+0.6xIV+XI			
205	Komb.: I+II+0.6xIII+0.6xIV+X			
206	Komb.: I+II+0.6xIII+0.6xIV+IX			
207	Komb.: I+II+0.6xIII+0.6xIV+VIII			
208	Komb.: 1.35xI+1.35xII+1.5xVII	-401.62	146.77	-1151.32
209	Komb.: 1.35xI+1.35xII+1.5xVI	-416.62	139.27	-1151.32
210	Komb.: 1.35xI+1.35xII+1.5xV	-401.62	139.27	-1217.76
211	Komb.: 1.35xI+1.35xII+1.5xIV	-401.62	139.27	-1253.19
212	Komb.: 1.35xI+1.35xII+1.5xIII	-405.37	135.52	-1166.32
213	Komb.: I+1.35xII+1.5xVII	-401.62	146.77	-852.83
214	Komb.: I+1.35xII+1.5xVI	-416.62	139.27	-852.83
215	Komb.: I+1.35xII+1.5xV	-401.62	139.27	-919.27
216	Komb.: I+1.35xII+1.5xIV	-401.62	139.27	-954.70
217	Komb.: I+1.35xII+1.5xIII	-405.37	135.52	-867.83
218	Komb.: 1.35xI+II+1.5xVII	-297.49	110.66	-1151.32
219	Komb.: 1.35xI+II+1.5xVI	-312.49	103.16	-1151.32
220	Komb.: 1.35xI+II+1.5xV	-297.49	103.16	-1217.76
221	Komb.: 1.35xI+II+1.5xIV	-297.49	103.16	-1253.19
222	Komb.: 1.35xI+II+1.5xIII	-301.24	99.41	-1166.32
223	Komb.: I+II+0.6xIV-1xVIII			
224	Komb.: I+II+0.6xIV-1xIX			
225	Komb.: I+II+0.6xIV-1xX			
226	Komb.: I+II+0.6xIV-1xXI			
227	Komb.: I+II+0.6xIV+XI			
228	Komb.: I+II+0.6xIV+X			
229	Komb.: I+II+0.6xIV+IX			
230	Komb.: I+II+0.6xIV+VIII			
231	Komb.: I+II+0.6xIII-1xVIII			
232	Komb.: I+II+0.6xIII-1xIX			
233	Komb.: I+II+0.6xIII-1xX			
234	Komb.: I+II+0.6xIII-1xXI			
235	Komb.: I+II+0.6xIII+XI			
236	Komb.: I+II+0.6xIII+X			
237	Komb.: I+II+0.6xIII+IX			
238	Komb.: I+II+0.6xIII+VIII			
239	Komb.: I+II+1.5xVII	-297.49	110.66	-852.83
240	Komb.: I+II+1.5xVI	-312.49	103.16	-852.83
241	Komb.: I+II+1.5xV	-297.49	103.16	-919.27
242	Komb.: I+II+1.5xIV	-297.49	103.16	-954.70
243	Komb.: I+II+1.5xIII	-301.24	99.41	-867.83
244	Komb.: I+II-1xVIII			
245	Komb.: I+II-1xIX			
246	Komb.: I+II-1xX			
247	Komb.: I+II-1xXI			
248	Komb.: I+II+XI			
249	Komb.: I+II+X			
250	Komb.: I+II+IX			
251	Komb.: I+II+VIII			
252	Komb.: 1.35xI+1.35xII	-401.62	139.27	-1151.32
253	Komb.: I+1.35xII	-401.62	139.27	-852.83
254	Komb.: 1.35xI+II	-297.49	103.16	-1151.32
255	Komb.: I+II	-297.49	103.16	-852.83
256	Komb.: I+II+0.7xIII+0.7xIV+V+0.6xVII	-299.24	104.41	-951.67
257	Komb.: I+II+0.7xIII+0.7xIV+V+0.6xVI	-305.24	101.41	-951.67
258	Komb.: I+II+0.7xIII+0.7xIV+0.5xV+VII	-299.24	106.41	-929.52
259	Komb.: I+II+0.7xIII+0.7xIV+0.5xV+VI	-309.24	101.41	-929.52
260	Komb.: I+II+0.7xIII+IV+0.5xV+0.6xVII	-299.24	104.41	-949.89
261	Komb.: I+II+0.7xIII+IV+0.5xV+0.6xVI	-305.24	101.41	-949.89
262	Komb.: I+II+III+0.7xIV+0.5xV+0.6xVII	-299.99	103.66	-932.52
263	Komb.: I+II+III+0.7xIV+0.5xV+0.6xVI	-305.99	100.66	-932.52
264	Komb.: I+II+0.7xIII+0.7xIV+VII	-299.24	106.41	-907.37
265	Komb.: I+II+0.7xIII+0.7xIV+VI	-309.24	101.41	-907.37
266	Komb.: I+II+0.7xIII+0.7xIV+V	-299.24	101.41	-951.67
267	Komb.: I+II+0.7xIV+V+0.6xVII	-297.49	106.16	-944.67
268	Komb.: I+II+0.7xIV+V+0.6xVI	-303.49	103.16	-944.67
269	Komb.: I+II+0.7xIII+V+0.6xVII	-299.24	104.41	-904.13
270	Komb.: I+II+0.7xIII+V+0.6xVI	-305.24	101.41	-904.13
271	Komb.: I+II+0.7xIII+IV+0.6xVII	-299.24	104.41	-927.75
272	Komb.: I+II+0.7xIII+IV+0.6xVI	-305.24	101.41	-927.75
273	Komb.: I+II+III+0.7xIV+0.6xVII	-299.99	103.66	-910.37
274	Komb.: I+II+III+0.7xIV+0.6xVI	-305.99	100.66	-910.37
275	Komb.: I+II+0.7xIV+0.5xV+VII	-297.49	108.16	-922.52
276	Komb.: I+II+0.7xIV+0.5xV+VI	-307.49	103.16	-922.52
277	Komb.: I+II+0.7xIII+0.5xV+VII	-299.24	106.41	-881.98
278	Komb.: I+II+0.7xIII+0.5xV+VI	-309.24	101.41	-881.98
279	Komb.: I+II+0.7xIII+IV+0.5xV	-299.24	101.41	-949.89
280	Komb.: I+II+III+0.7xIV+0.5xV	-299.99	100.66	-932.52
281	Komb.: I+II+IV+0.5xV+0.6xVII	-297.49	106.16	-942.89
282	Komb.: I+II+IV+0.5xV+0.6xVI	-303.49	103.16	-942.89
283	Komb.: I+II+III+0.5xV+0.6xVII	-299.99	103.66	-884.98
284	Komb.: I+II+III+0.5xV+0.6xVI	-305.99	100.66	-884.98
285	Komb.: I+II+0.7xIV+VII	-297.49	108.16	-900.37
286	Komb.: I+II+0.7xIV+VI	-307.49	103.16	-900.37
287	Komb.: I+II+0.7xIV+V	-297.49	103.16	-944.67
288	Komb.: I+II+0.7xIII+VII	-299.24	106.41	-859.83
289	Komb.: I+II+0.7xIII+VI	-309.24	101.41	-859.83
290	Komb.: I+II+0.7xIII+V	-299.24	101.41	-904.13
291	Komb.: I+II+0.7xIII+IV	-299.24	101.41	-927.75
292	Komb.: I+II+III+0.7xIV	-299.99	100.66	-910.37
293	Komb.: I+II+V+0.6xVII	-297.49	106.16	-897.13
294	Komb.: I+II+V+0.6xVI	-303.49	103.16	-897.13
295	Komb.: I+II+IV+0.6xVII	-297.49	106.16	-920.75
296	Komb.: I+II+IV+0.6xVI	-303.49	103.16	-920.75
297	Komb.: I+II+III+0.6xVII	-299.99	103.66	-862.83
298	Komb.: I+II+III+0.6xVI	-305.99	100.66	-862.83
299	Komb.: I+II+0.5xV+VII	-297.49	108.16	-874.98
300	Komb.: I+II+0.5xV+VI	-307.49	103.16	-874.98
301	Komb.: I+II+IV+0.5xV	-297.49	103.16	-942.89
302	Komb.: I+II+III+0.5xV	-299.99	100.66	-884.98
303	Komb.: I+II+VII	-297.49	108.16	-852.83

Lista obtežnih primerov				
LC	Naziv	pX [kN]	pY [kN]	pZ [kN]
304	Komb.: I+II+VI	-307.49	103.16	-852.83
305	Komb.: I+II+V	-297.49	103.16	-897.13
306	Komb.: I+II+IV	-297.49	103.16	-920.75
307	Komb.: I+II+III	-299.99	100.66	-862.83
308	Komb.: I+II	-297.49	103.16	-852.83
309	Komb.: MSU (T=fin_1) (1.9xl+1.24xV+ +0.6xVI)	-6.00	0.00	-1675.30
310	Komb.: MSU (T=fin_2) (1.9xl+1.24xV+ +0.6xVII)	-0.00	3.00	-1675.30
311	Komb.: MSU (T=fin_3) (1.9xl+ +1.27xVI)	-12.70	0.00	-1620.37
312	Komb.: MSU (T=fin_4) (1.9xl+ +1.27xVII)	-0.00	6.35	-1620.37

Seznam obtežnih primerov ovojnic	
LC	Naziv
313	[MSN] 12-199,208-222,239-243,252-254
314	[POTRES] 200-207,223-238,244-251

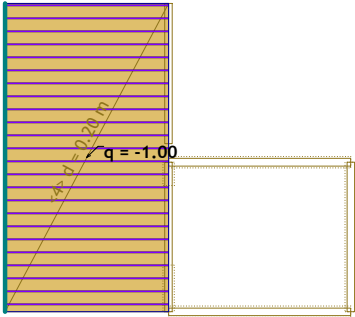
Obt. 1: Lastna teža (g)



Nivo: [8.01 m]

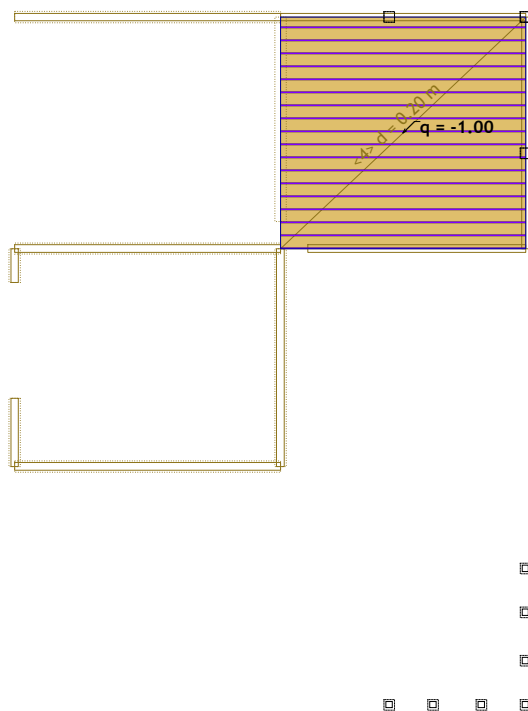
LC	Naziv
315	[MSU (T=0)] 255-308
316	[msu (t=fin)] 309-312

Obt. 1: Lastna teža (g)

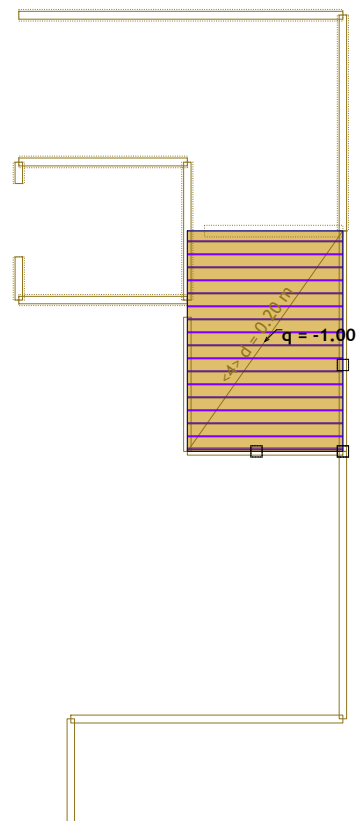


Nivo: [4.33 m]

Obt. 1: Lastna teža (g)

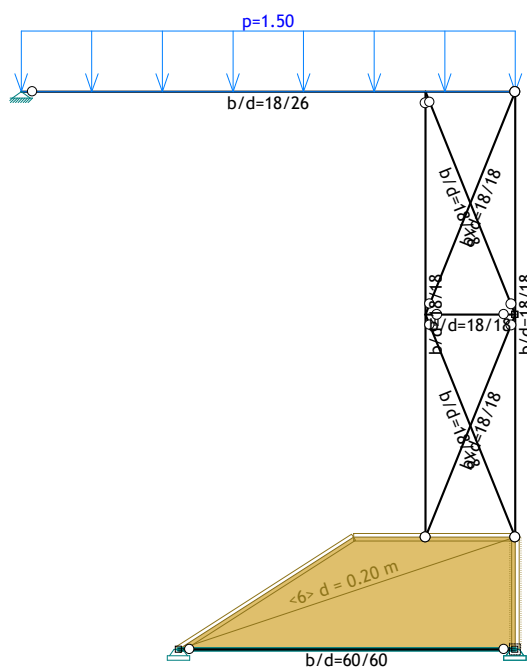


Obt. 1: Lastna teža (g)



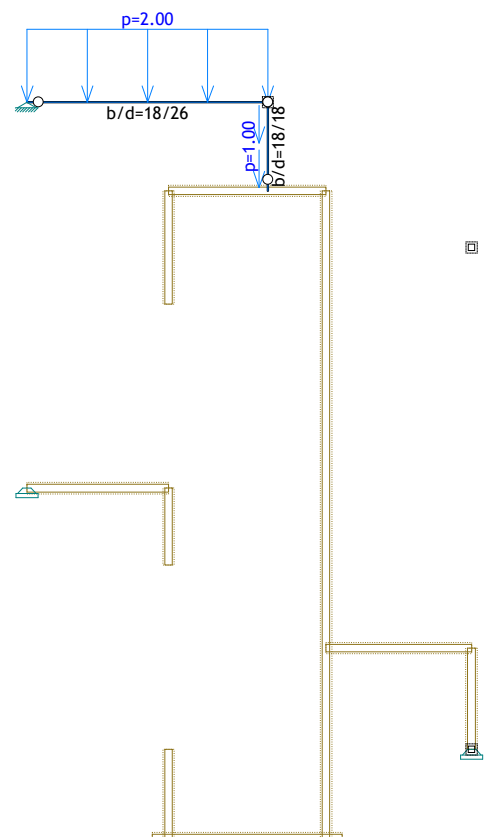
Nivo: [3.15 m]

Obt. 1: Lastna teža (g)



Nivo: [2.35 m]

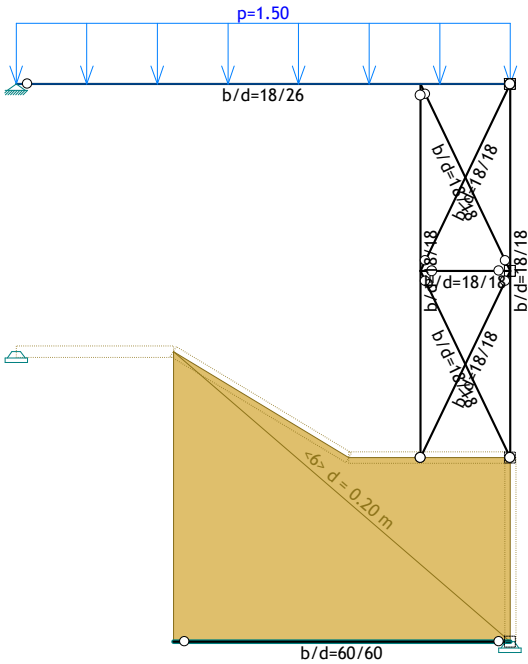
Obt. 1: Lastna teža (g)



Okvir: H_3

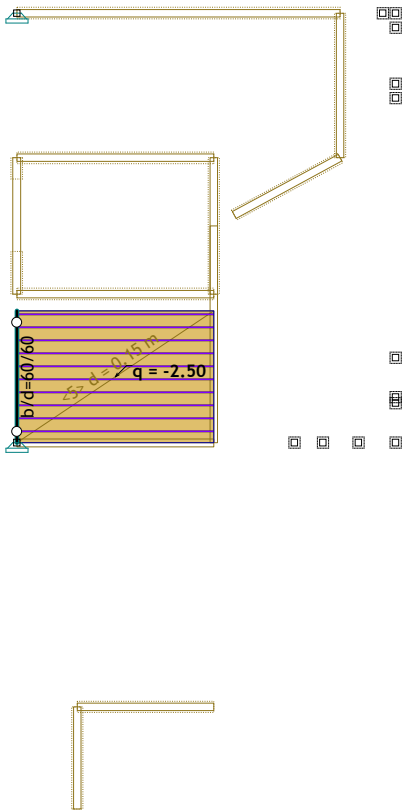
Okvir: H_6

Obt. 1: Lastna teža (g)



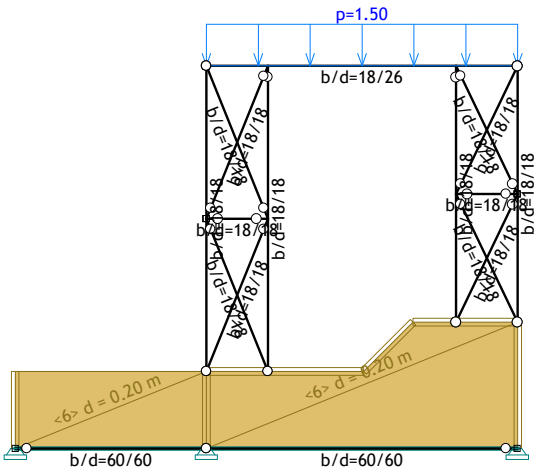
Okvir: H_4

Obt. 1: Lastna teža (g)



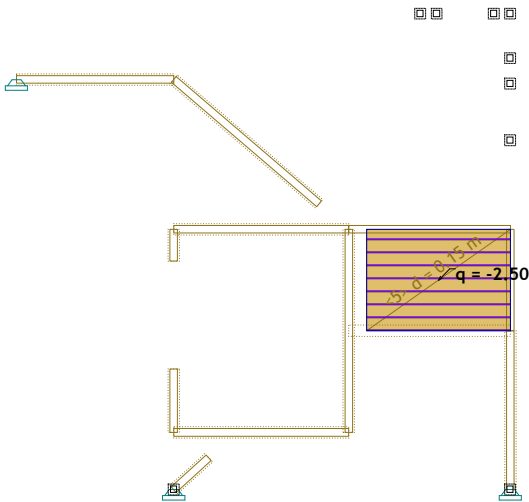
Pogled: Rama_1

Obt. 1: Lastna teža (g)



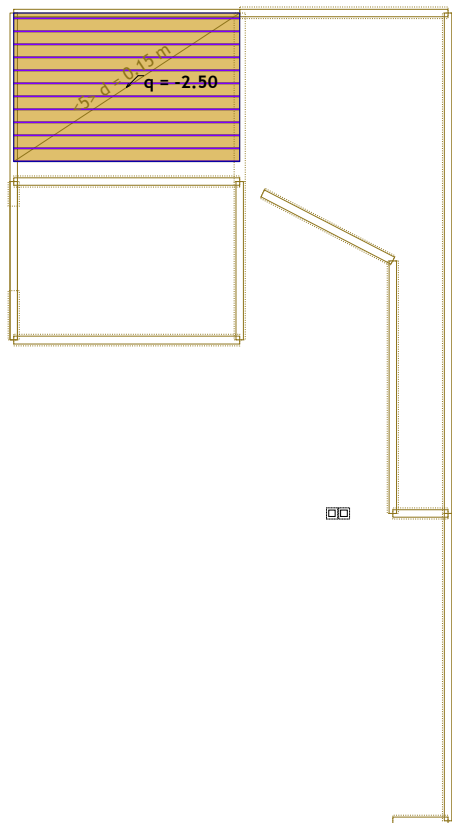
Okvir: V_3

Obt. 1: Lastna teža (g)

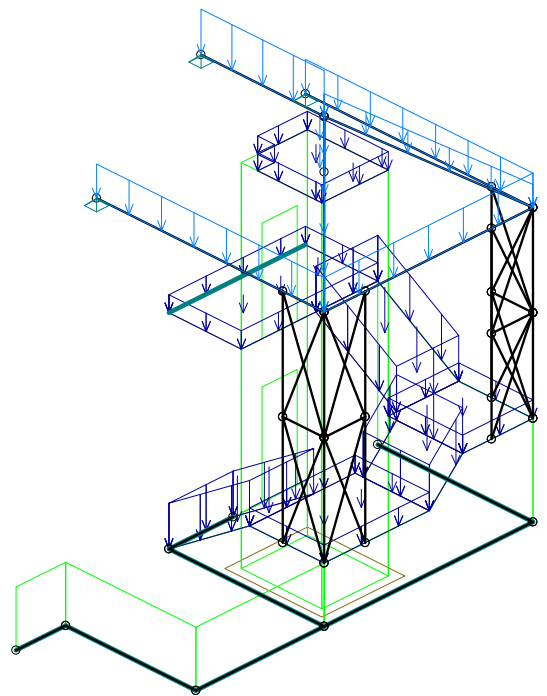


Pogled: Rama_2

Obt. 1: Lastna teža (g)

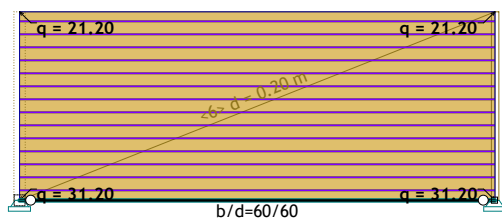


Obt. 1: Lastna teža (g)



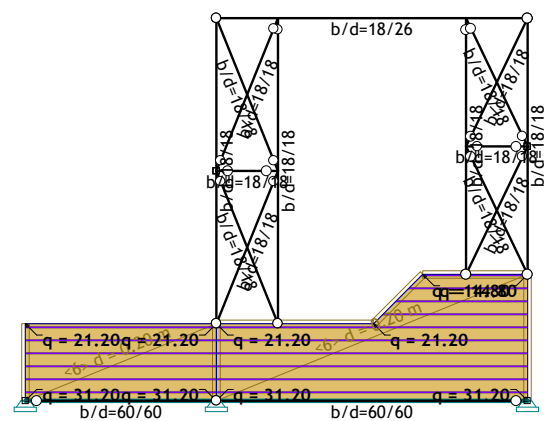
Pogled: Rama_3

Obt. 2: Lastna teža - pritisak zemljine



Izometrija

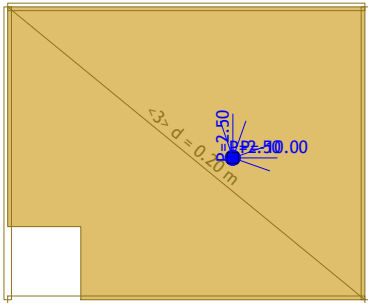
Obt. 2: Lastna teža - pritisak zemljine



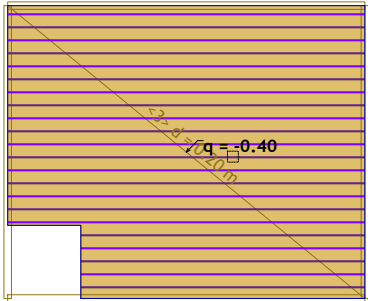
Okvir: H_5

Okvir: V_3

Obt. 3: Obtežba dvigalo

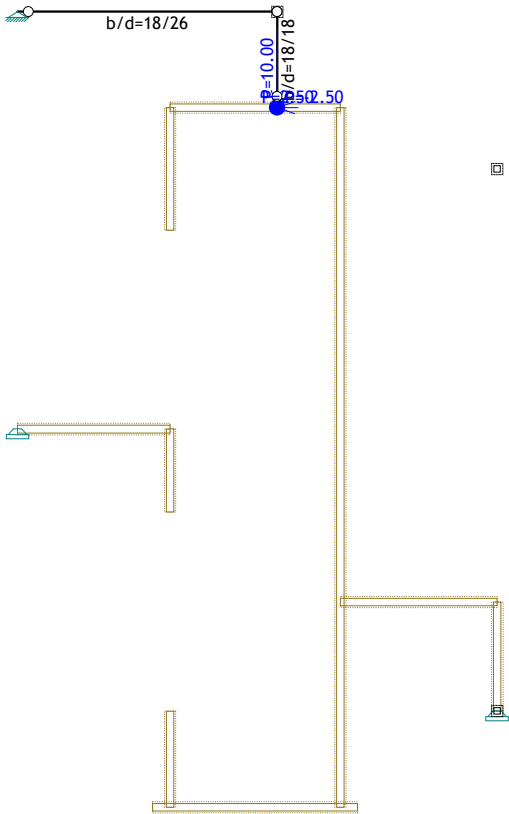


Nivo: [8.01 m]
Obt. 4: Koristna teža

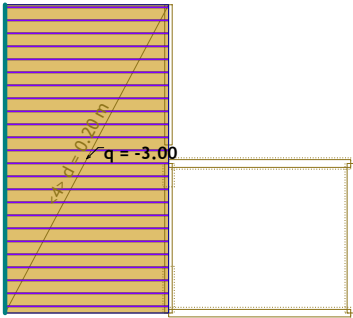


Nivo: [8.01 m]

Obt. 3: Obtežba dvigalo

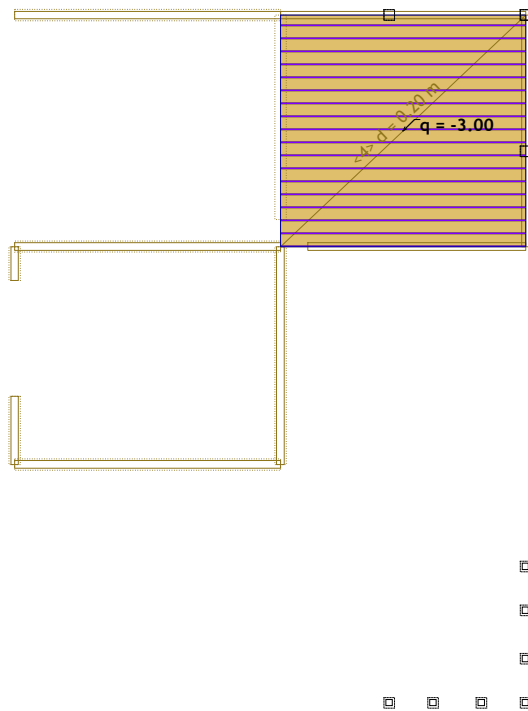


Okvir: H_6
Obt. 4: Koristna teža

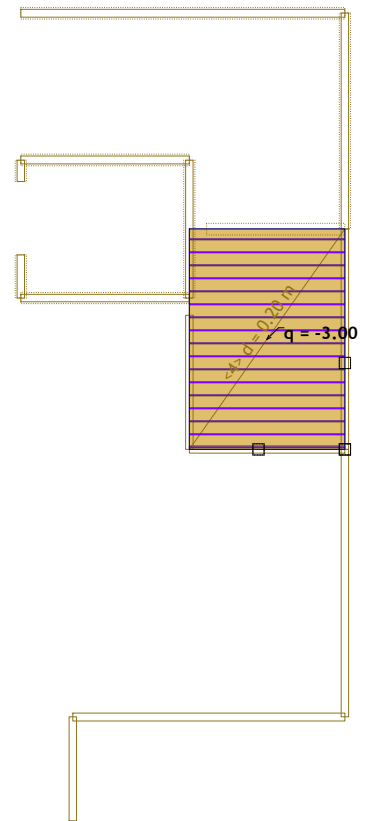


Nivo: [4.33 m]

Obt. 4: Koristna teža

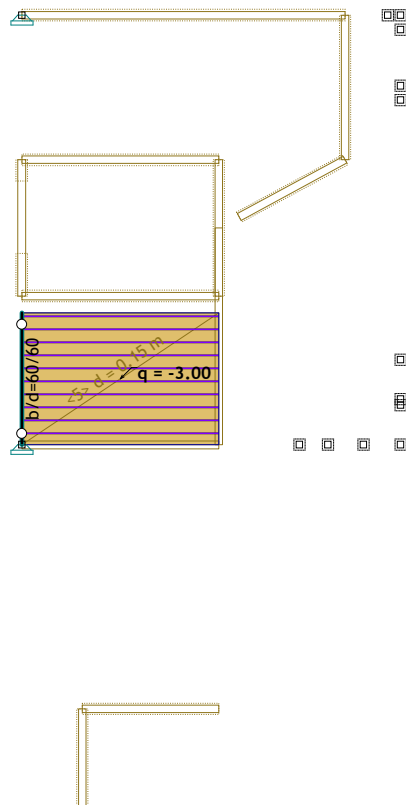


Obt. 4: Koristna teža



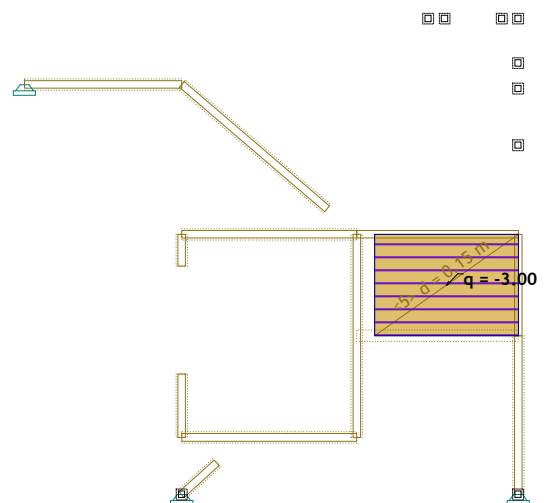
Nivo: [3.15 m]

Obt. 4: Koristna teža



Nivo: [2.35 m]

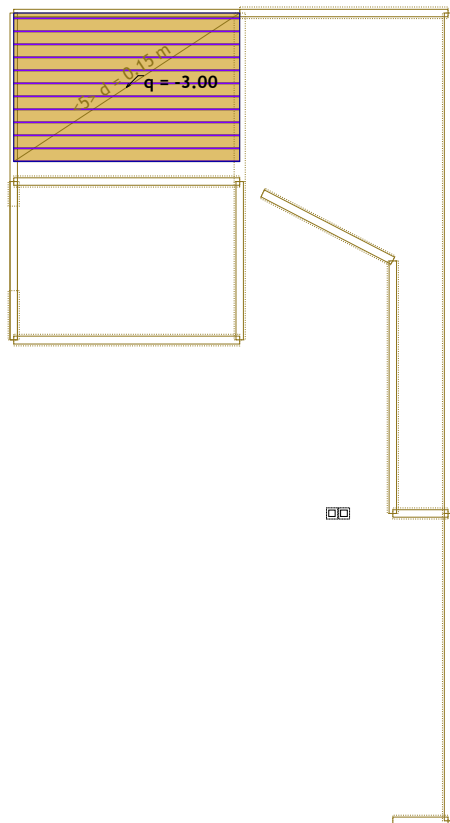
Obt. 4: Koristna teža



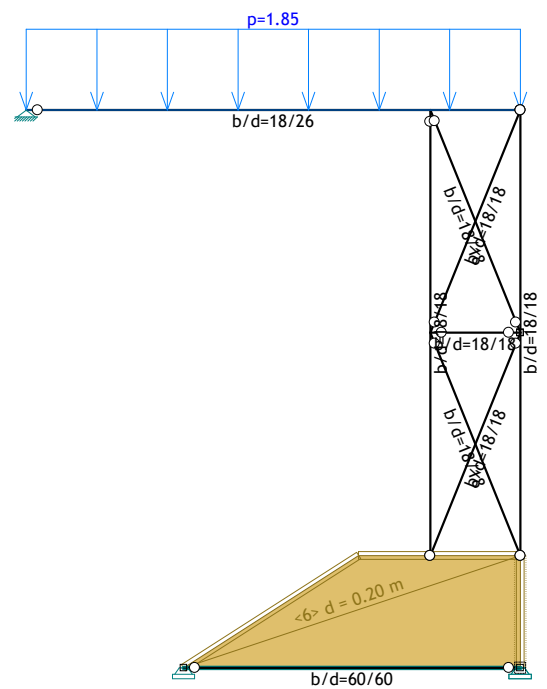
Pogled: Rama_1

Pogled: Rama_2

Obt. 4: Koristna teža

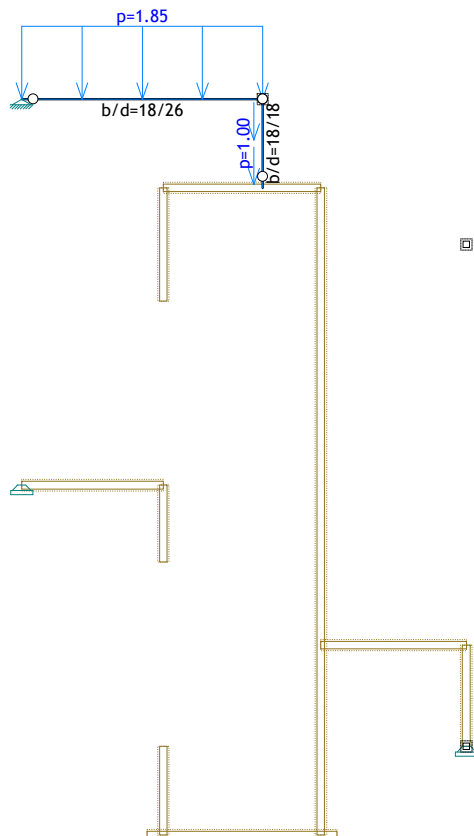


Obt. 5: Sneg



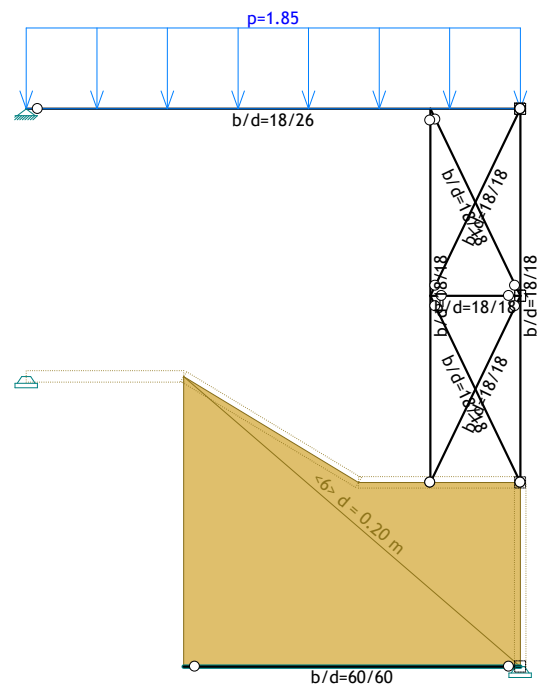
Pogled: Rama_3

Obt. 5: Sneg



Okvir: H_3

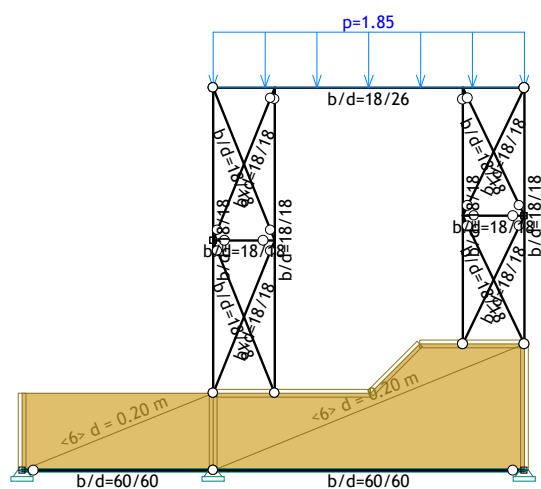
Obt. 5: Sneg



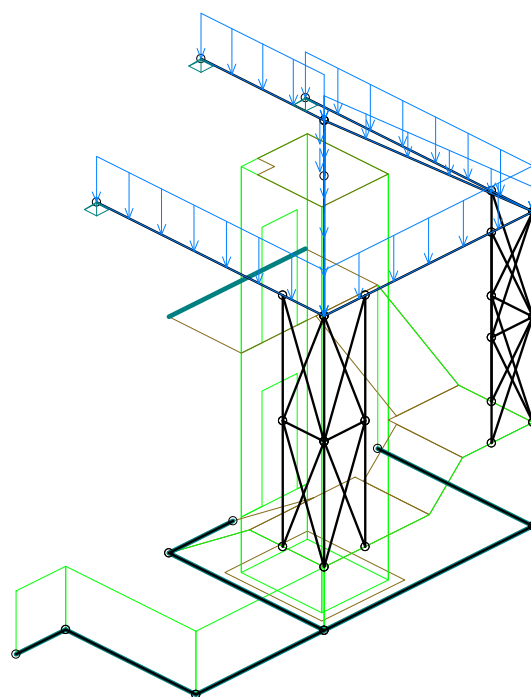
Okvir: H_6

Okvir: H_4

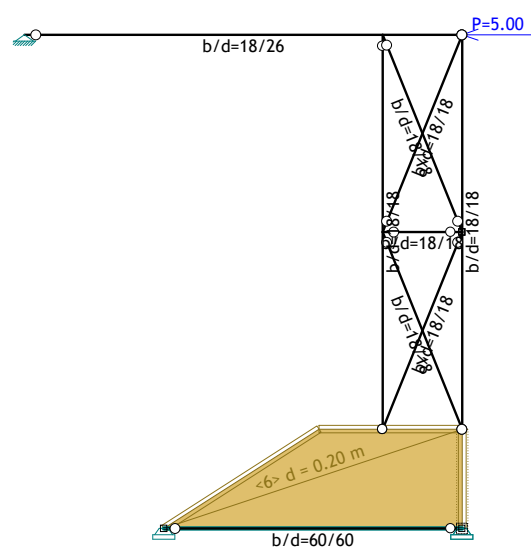
Obt. 5: Sneg



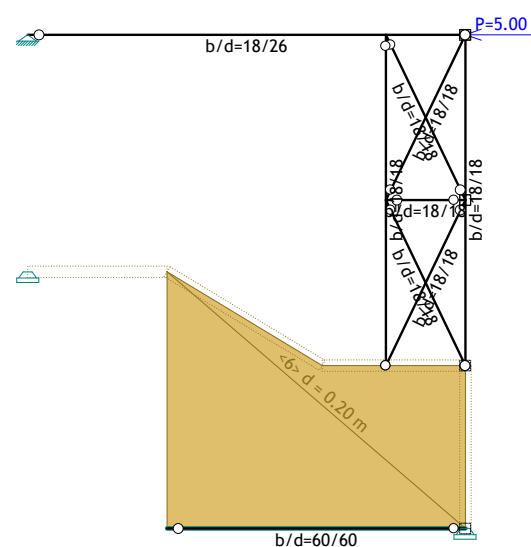
Obt. 5: Sneg



Okvir: V_3
Obt. 6: Veter_x



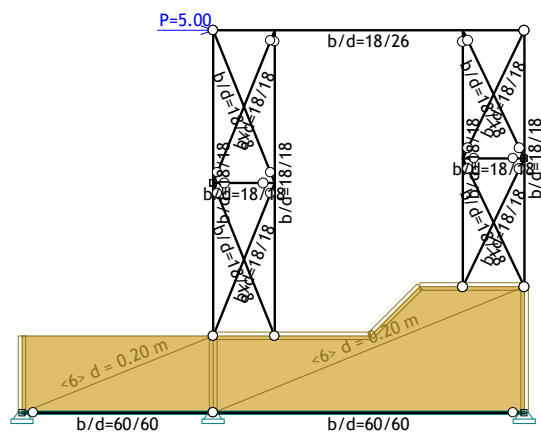
Izometrija
Obt. 6: Veter_x



Okvir: H_3

Okvir: H_4

Obt. 7: Veter_y



Okvir: V_3

Napredne opcije seizmičnega preračuna

Zidovi - redukcija togosti na upogib:	0.500
Zidovi - redukcija osne togosti:	0.500
Stebri - redukcija upogibne togosti:	0.500
Stebri - redukcija osne togosti:	0.500
Sodelovanje zidov:	6.000 x d
Preprečeno nihanje v Z smeri	

Faktorji obtežb za preračun mas

No	Naziv	Koeficient
1	Lastna teža (g)	1.00
2	Lastna teža - pritisk zemljine	1.00
3	Obtežba dvigalo	0.30
4	Koristna teža	0.30
5	Sneg	0.00
6	Veter x	0.00
7	Veter y	0.00

Razporeditev mas po višini objekta

Nivo	Z [m]	X [m]	Y [m]	Masa [T]	T/m²
	8.01	1.36	0.80	13.36	4.51
	4.33	0.57	1.22	14.29	2.47
	3.15	1.92	1.84	9.13	2.98
	2.35	2.31	-0.18	15.68	3.42
	1.10	2.20	-0.83	31.31	
	0.00	0.98	0.80	5.59	1.19
Skupno:	3.01	1.73	0.23	89.35	

Položaj centra togosti po višini objekta (približna metoda)

Nivo	Z [m]	X [m]	Y [m]
	8.01	0.98	0.80
	4.33	1.07	0.80
	3.15	2.43	1.81
	2.35	3.44	1.21
	1.10	3.48	-0.28
	0.00	0.98	0.80

Ekscentriciteta po višini objekta (približna metoda)

Nivo	Z [m]	eox [m]	eoy [m]
	8.01	0.38	0.00
	4.33	0.50	0.42
	3.15	0.52	0.03
	2.35	1.14	1.39
	1.10	1.28	0.55
	0.00	0.00	0.00

Nihajne dobe konstrukcije

No	T [s]	f [Hz]	m' [T]
1	0.1913	5.2279	2.9214
2	0.1445	6.9220	2.5746
3	0.1372	7.2877	8.7559
4	0.1363	7.3371	1.5046
5	0.1239	8.0728	15.9240
6	0.1043	9.5887	4.7304
7	0.0748	13.3603	1.1705
8	0.0713	14.0185	0.3463
9	0.0655	15.2636	13.9106
10	0.0650	15.3820	2.9647

Seizmični preračun

Seizmični preračun: EC8 (SIST EN 1998)

Kategorija tal:	C
Kategorija pomena:	II ($\gamma=1.0$)
Razmerje $a_g R/g$:	0.250
Koeficient dušenja:	0.05
Slučajna ekscentričnost mase etaže:	$e_i = \pm 0.050 \times L_i$

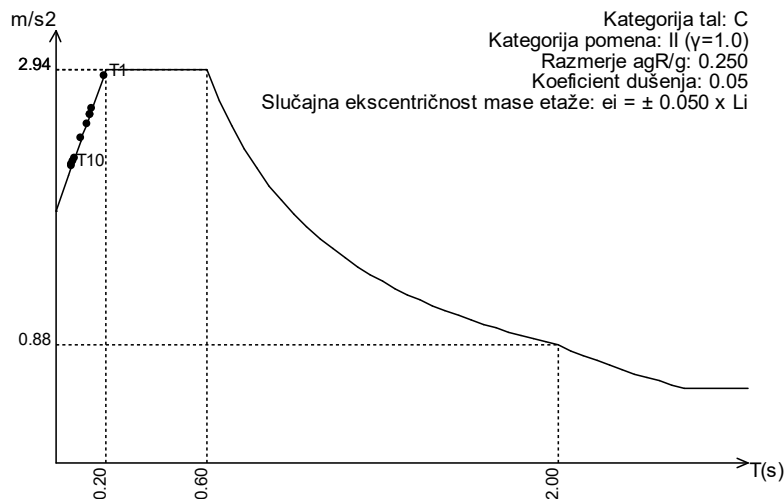
Faktorji smeri potresa:

Obtežni primer	Kot α [°]	k, α	$k, \alpha+90^\circ$	k_z	Faktor O_i
Potres x	0	1.000	0.300	0.000	2.400*
Potres y	90	1.000	0.300	0.000	2.400*

Tip spektra

Obtežni primer	S	T_b	T_c	T_d	avg/ag
Potres x	1.150	0.200	0.600	2.000	1.000
Potres y	1.150	0.200	0.600	2.000	1.000

Projektni spekter



S=1.15, $T_b=0.20$, $T_c=0.60$, $T_d=2.00$

Razporeditev potresnih sil po višini objekta - Potres x (+e)

Konstrukcija neregularna po višini, Duktilni sistemi povezanih sten (Sistem zidov: Samo dva neodvisna zida v eni smeri - $\alpha u/\alpha_1=1$), Razred duktilnosti DCM:
 $q_0=0.8 \times 3\alpha u/\alpha_1=2.40$

Okrvni v dvojni dominantno okvirni sistem: $\alpha_0=2.00$, $k_w=1.00$.

Faktor obnašanja: $q=q_0 \times k_w=2.40$

Nivo	Z [m]	Ton 1			Ton 2			Ton 3		
		Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]
	8.01	0.52	8.41	-0.26	-0.01	0.02	-0.00	22.51	13.93	-0.80
	4.33	0.32	2.09	-0.37	0.00	0.01	-0.00	16.11	13.73	0.71
	3.15	-0.00	0.83	-0.51	0.00	0.00	-0.00	5.50	5.73	-2.80
	2.35	0.05	0.94	-0.10	0.00	0.00	-0.00	5.20	6.06	-1.65
	1.10	0.03	0.82	0.12	0.00	0.00	0.00	4.54	5.86	-1.34
	0.00	-0.09	-0.12	-0.08	-0.00	-0.00	-0.00	-2.26	-1.74	0.30
	$\Sigma=$	0.83	12.97	-1.20	-0.01	0.04	-0.00	51.60	43.57	-5.58

Nivo	Z [m]	Ton 4			Ton 5			Ton 6		
		Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]
	8.01	-0.60	0.90	-0.04	12.66	-11.28	0.56	0.05	-0.08	-0.02
	4.33	0.78	0.92	0.05	12.65	-9.69	3.36	0.07	-0.30	0.01
	3.15	0.31	0.38	-0.18	5.18	-3.42	1.05	0.02	-0.07	-0.01
	2.35	0.30	0.39	-0.10	5.33	-3.45	-1.31	0.05	-0.04	-0.02
	1.10	0.26	0.39	-0.09	4.14	-4.14	-2.09	0.02	-0.08	-0.01
	0.00	-0.16	-0.12	0.03	-0.34	1.60	0.65	0.03	0.08	-0.01
	$\Sigma=$	0.89	2.85	-0.32	39.63	-30.38	2.24	0.26	-0.49	-0.06

Nivo	Z [m]	Ton 7			Ton 8			Ton 9		
		Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]
	8.01	-0.20	-0.14	-0.02	-0.06	0.12	0.00	-5.16	-5.51	-1.43
	4.33	0.20	0.16	-0.03	0.08	-0.13	-0.01	3.11	7.00	-1.97
	3.15	0.05	0.07	-0.01	0.03	-0.06	-0.00	5.34	7.18	0.42
	2.35	0.08	0.09	0.00	0.05	-0.08	0.01	7.69	13.62	0.33
	1.10	0.12	0.16	0.02	0.08	-0.14	0.01	12.86	24.68	1.90
	0.00	0.06	0.04	-0.01	0.03	-0.06	0.00	6.59	9.53	-0.92
	$\Sigma=$	0.31	0.37	-0.04	0.21	-0.34	0.01	30.43	56.49	-1.68

Nivo	Z [m]	Ton 10		
		Px [kN]	Py [kN]	Pz [kN]
	8.01	-4.97	4.71	-0.01
	4.33	3.32	0.46	-1.59
	3.15	5.90	-3.48	0.13
	2.35	8.54	-7.85	1.22
	1.10	15.69	-13.95	1.55
	0.00	6.13	-3.88	-0.13
	$\Sigma=$	34.60	-23.98	1.16

Razporeditev potresnih sil po višini objekta - Potres x (-e)

Konstrukcija neregularna po višini, Duktilni sistemi povezanih sten (Sistem zidov: Samo dva neodvisna zida v eni smeri - $\alpha u/\alpha 1=1$), Razred duktilnosti DCM:

$q_0=0.8 \cdot 3\alpha u/\alpha 1=2.40$

Okvirni in dvojni dominantno okvirni sistem: $\alpha_0=2.00$, $k_w=1.00$.

Faktor obnašanja: $q=q_0 \cdot k_w=2.40$

Nivo	Z [m]	Ton 1			Ton 2			Ton 3		
		Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]
	8.01	0.52	8.41	-0.26	-0.01	0.02	-0.00	22.51	13.93	-0.80
	4.33	0.32	2.09	-0.37	0.00	0.01	-0.00	16.11	13.73	0.71
	3.15	-0.00	0.83	-0.51	0.00	0.00	-0.00	5.50	5.73	-2.80
	2.35	0.05	0.94	-0.10	0.00	0.00	-0.00	5.20	6.06	-1.65
	1.10	0.03	0.82	0.12	0.00	0.00	0.00	4.54	5.86	-1.34
	0.00	-0.09	-0.12	-0.08	-0.00	-0.00	-0.00	-2.26	-1.74	0.30
	$\Sigma=$	0.83	12.97	-1.20	-0.01	0.04	-0.00	51.60	43.57	-5.58

Nivo	Z [m]	Ton 4			Ton 5			Ton 6		
		Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]
	8.01	-0.60	0.90	-0.04	12.66	-11.28	0.56	0.05	-0.08	-0.02
	4.33	0.78	0.92	0.05	12.65	-9.69	3.36	0.07	-0.30	0.01
	3.15	0.31	0.38	-0.18	5.18	-3.42	1.05	0.02	-0.07	-0.01
	2.35	0.30	0.39	-0.10	5.33	-3.45	-1.31	0.05	-0.04	-0.02
	1.10	0.26	0.39	-0.09	4.14	-4.14	-2.09	0.02	-0.08	-0.01
	0.00	-0.16	-0.12	0.03	-0.34	1.60	0.65	0.03	0.08	-0.01
	$\Sigma=$	0.89	2.85	-0.32	39.63	-30.38	2.24	0.26	-0.49	-0.06

Nivo	Z [m]	Ton 7			Ton 8			Ton 9		
		Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]
	8.01	-0.20	-0.14	-0.02	-0.06	0.12	0.00	-5.16	-5.51	-1.43
	4.33	0.20	0.16	-0.03	0.08	-0.13	-0.01	3.11	7.00	-1.97
	3.15	0.05	0.07	-0.01	0.03	-0.06	-0.00	5.34	7.18	0.42
	2.35	0.08	0.09	0.00	0.05	-0.08	0.01	7.69	13.62	0.33
	1.10	0.12	0.16	0.02	0.08	-0.14	0.01	12.86	24.68	1.90
	0.00	0.06	0.04	-0.01	0.03	-0.06	0.00	6.59	9.53	-0.92
	$\Sigma=$	0.31	0.37	-0.04	0.21	-0.34	0.01	30.43	56.49	-1.68

Nivo	Z [m]	Ton 10		
		Px [kN]	Py [kN]	Pz [kN]
	8.01	-4.97	4.71	-0.01
	4.33	3.32	0.46	-1.59
	3.15	5.90	-3.48	0.13
	2.35	8.54	-7.85	1.22
	1.10	15.69	-13.95	1.55
	0.00	6.13	-3.88	-0.13
	$\Sigma=$	34.60	-23.98	1.16

Razporeditev potresnih sil po višini objekta - Potres y (+e)

Konstrukcija neregularna po višini, Duktilni sistemi povezanih sten (Sistem zidov: Samo dva neodvisna zida v eni smeri - $\alpha u/\alpha 1=1$), Razred duktilnosti DCM:

$q_0=0.8 \cdot 3\alpha u/\alpha 1=2.40$

Okvirni in dvojni dominantno okvirni sistem: $\alpha_0=2.00$, $k_w=1.00$.

Faktor obnašanja: $q=q_0 \cdot k_w=2.40$

Nivo	Z [m]	Ton 1			Ton 2			Ton 3		
		Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]
	8.01	1.40	22.65	-0.70	-0.22	0.37	-0.02	9.78	6.05	-0.35
	4.33	0.86	5.62	-1.00	0.02	0.19	-0.03	7.00	5.97	0.31
	3.15	-0.00	2.24	-1.38	0.01	0.08	-0.04	2.39	2.49	-1.21
	2.35	0.13	2.53	-0.27	0.01	0.10	-0.01	2.26	2.63	-0.72
	1.10	0.09	2.21	0.33	0.01	0.09	0.01	1.97	2.54	-0.58
	0.00	-0.25	-0.31	-0.22	-0.02	-0.03	-0.00	-0.98	-0.76	0.13
	$\Sigma=$	2.24	34.94	-3.24	-0.20	0.81	-0.10	22.42	18.93	-2.42

Nivo	Z [m]	Ton 4			Ton 5			Ton 6		
		Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]
	8.01	-0.90	1.33	-0.05	-17.55	15.63	-0.78	-0.27	0.40	0.12
	4.33	1.15	1.37	0.08	-17.52	13.43	-4.66	-0.39	1.55	-0.04
	3.15	0.46	0.56	-0.26	-7.17	4.73	-1.46	-0.12	0.39	0.06
	2.35	0.44	0.58	-0.15	-7.39	4.78	1.81	-0.27	0.23	0.12
	1.10	0.38	0.57	-0.14	-5.74	5.74	2.89	-0.13	0.42	0.03
	0.00	-0.23	-0.18	0.04	0.47	-2.21	-0.91	-0.16	-0.43	0.03
	$\Sigma=$	1.32	4.23	-0.48	-54.90	42.10	-3.10	-1.34	2.56	0.31

Nivo	Z [m]	Ton 7			Ton 8			Ton 9		
		Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]
	8.01	-0.13	-0.09	-0.01	0.24	-0.47	-0.01	-5.16	-5.51	-1.43
	4.33	0.13	0.10	-0.02	-0.30	0.51	0.02	3.11	7.00	-1.97
	3.15	0.03	0.04	-0.00	-0.13	0.22	0.00	5.34	7.18	0.42
	2.35	0.05	0.06	0.00	-0.18	0.30	-0.03	7.69	13.61	0.33
	1.10	0.08	0.10	0.01	-0.32	0.53	-0.03	12.86	24.67	1.90
	0.00	0.04	0.02	-0.01	-0.11	0.22	-0.01	6.59	9.53	-0.92
	$\Sigma=$	0.20	0.24	-0.02	-0.80	1.31	-0.05	30.42	56.48	-1.68

Nivo	Z [m]	Ton 10		
		Px [kN]	Py [kN]	Pz [kN]
	8.01	6.23	-5.91	0.01
	4.33	-4.16	-0.58	2.00
	3.15	-7.40	4.36	-0.16
	2.35	-10.70	9.84	-1.54
	1.10	-19.67	17.48	-1.94
	0.00	-7.68	4.86	0.17
	$\Sigma=$	-43.37	30.06	-1.45

Razporeditev potresnih sil po višini objekta - Potres y (-e)

Konstrukcija neregularna po višini, Duktilni sistemi povezanih sten (Sistem zidov: Samo dva neodvisna zida v eni smeri - $\alpha u/\alpha 1=1$), Razred duktilnosti DCM:

$q_0=0.8 \cdot 3\alpha u/\alpha 1=2.40$

Okvirni in dvojni dominantno okvirni sistem: $\alpha_0=2.00$, $k_w=1.00$.

Faktor obnašanja: $q=q_0 \cdot k_w=2.40$

Nivo	Z [m]	Ton 1			Ton 2			Ton 3		
		Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]
	8.01	1.40	22.65	-0.70	-0.22	0.37	-0.02	9.78	6.05	-0.35
	4.33	0.86	5.62	-1.00	0.02	0.19	-0.03	7.00	5.97	0.31
	3.15	-0.00	2.24	-1.38	0.01	0.08	-0.04	2.39	2.49	-1.21
	2.35	0.13	2.53	-0.27	0.01	0.10	-0.01	2.26	2.63	-0.72
	1.10	0.09	2.21	0.33	0.01	0.09	0.01	1.97	2.54	-0.58
	0.00	-0.25	-0.31	-0.22	-0.02	-0.03	-0.00	-0.98	-0.76	0.13
	Σ=	2.24	34.94	-3.24	-0.20	0.81	-0.10	22.42	18.93	-2.42

Nivo	Z [m]	Ton 4			Ton 5			Ton 6		
		Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]
	8.01	-0.90	1.33	-0.05	-17.55	15.63	-0.78	-0.27	0.40	0.12
	4.33	1.15	1.37	0.08	-17.52	13.43	-4.66	-0.39	1.55	-0.04
	3.15	0.46	0.56	-0.26	-7.17	4.73	-1.46	-0.12	0.39	0.06
	2.35	0.44	0.58	-0.15	-7.39	4.78	1.81	-0.27	0.23	0.12
	1.10	0.38	0.57	-0.14	-5.74	5.74	2.89	-0.13	0.42	0.03
	0.00	-0.23	-0.18	0.04	0.47	-2.21	-0.91	-0.16	-0.43	0.03
	Σ=	1.32	4.23	-0.48	-54.90	42.10	-3.10	-1.34	2.56	0.31

Nivo	Z [m]	Ton 7			Ton 8			Ton 9		
		Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]
	8.01	-0.13	-0.09	-0.01	0.24	-0.47	-0.01	-5.16	-5.51	-1.43
	4.33	0.13	0.10	-0.02	-0.30	0.51	0.02	3.11	7.00	-1.97
	3.15	0.03	0.04	-0.00	-0.13	0.22	0.00	5.34	7.18	0.42
	2.35	0.05	0.06	0.00	-0.18	0.30	-0.03	7.69	13.61	0.33
	1.10	0.08	0.10	0.01	-0.32	0.53	-0.03	12.86	24.67	1.90
	0.00	0.04	0.02	-0.01	-0.11	0.22	-0.01	6.59	9.53	-0.92
	Σ=	0.20	0.24	-0.02	-0.80	1.31	-0.05	30.42	56.48	-1.68

Nivo	Z [m]	Ton 10		
		Px [kN]	Py [kN]	Pz [kN]
	8.01	6.23	-5.91	0.01
	4.33	-4.16	-0.58	2.00
	3.15	-7.40	4.36	-0.16
	2.35	-10.70	9.84	-1.54
	1.10	-19.67	17.48	-1.94
	0.00	-7.68	4.86	0.17
	Σ=	-43.37	30.06	-1.45

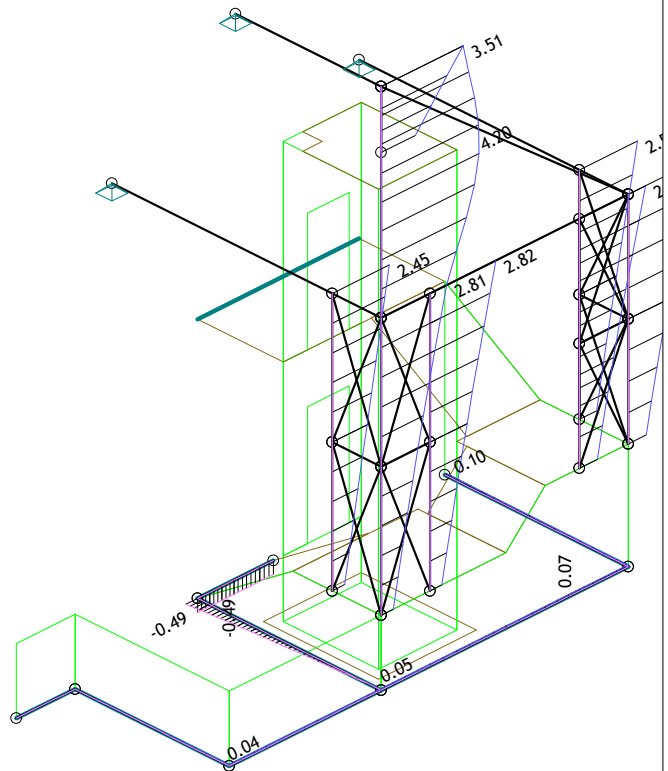
Faktorji participacije - relativno sodelovanje

Ton \ Naziv	1. Potres x	2. Potres x	3. Potres y	4. Potres y
1	0.027	0.027	0.167	0.167
2	0.000	0.000	0.004	0.004
3	0.365	0.365	0.060	0.060
4	0.010	0.010	0.019	0.019
5	0.172	0.172	0.286	0.286
6	0.001	0.001	0.014	0.014
7	0.002	0.002	0.001	0.001
8	0.001	0.001	0.008	0.008
9	0.268	0.268	0.231	0.231
10	0.155	0.155	0.210	0.210

Faktorji participacije - angažiranje mase

Ton	U [α=0°]	U [α=90°]
Upošteva se samo masa nad koto temelja		
Kota temelja: 1.50 m		
Skupna masa nad temeljem: 56.19 T		
Skupna masa celega objekta: 89.35 T		
1	0.10	20.60
2	0.03	0.44
3	29.12	18.97
4	0.27	2.79

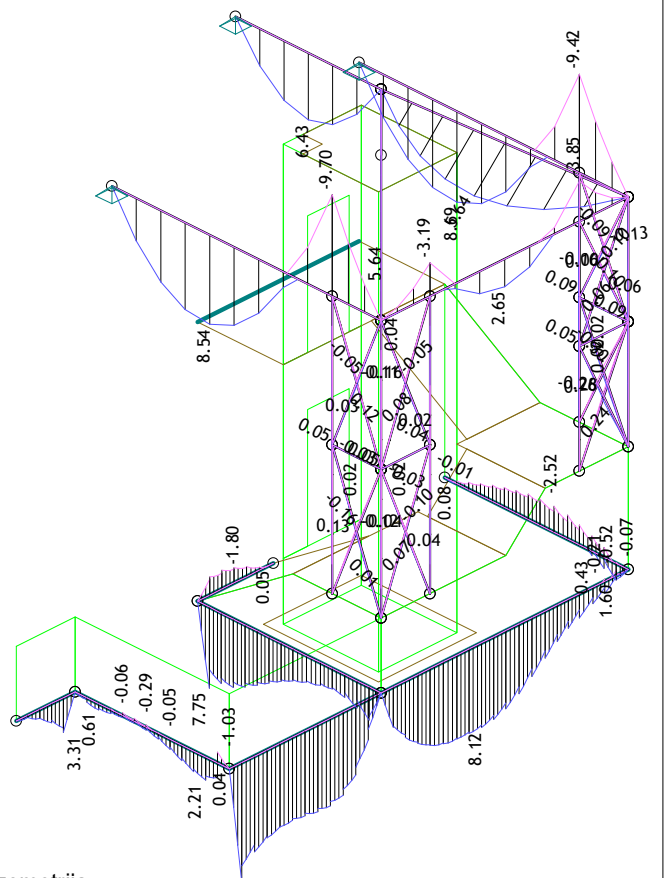
Ton	U [α=0°]	U [α=90°]
5	34.56	20.82
6	0.30	1.72
7	0.04	0.07
8	0.09	0.20
9	6.16	24.85
10	12.70	3.62
ΣU (%)	83.37	94.10



Izometrija

Vplivi v gredi: $\max Y_p = 4.20$ / $\min Y_p = -0.49$ m / 1000

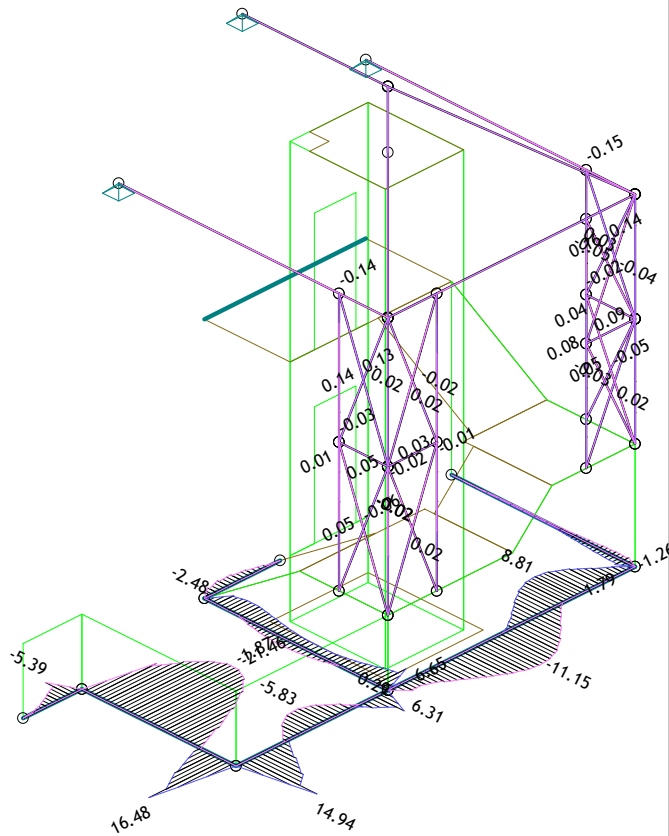
Obt. 313: [MSN] 12-199,208-222,239-243,252-254



Izometrija

Vplivi v gredi: $\max M_3 = 1.49$ / $\min M_3 = -9.70 \text{ kNm}$

Obt. 313: [MSN] 12-199,208-222,239-243,252-254

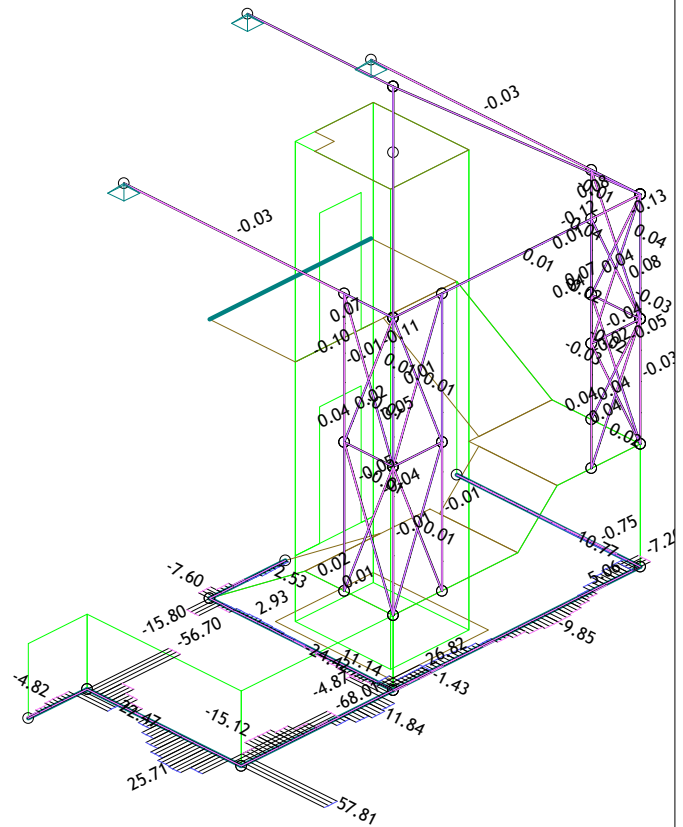


Izometrija

Vplivi v gredi: max M2= 16.48 / min M2= -21.46 kNm

Obt. 313: [MSN] 12-199,208-222,239-243,252-254

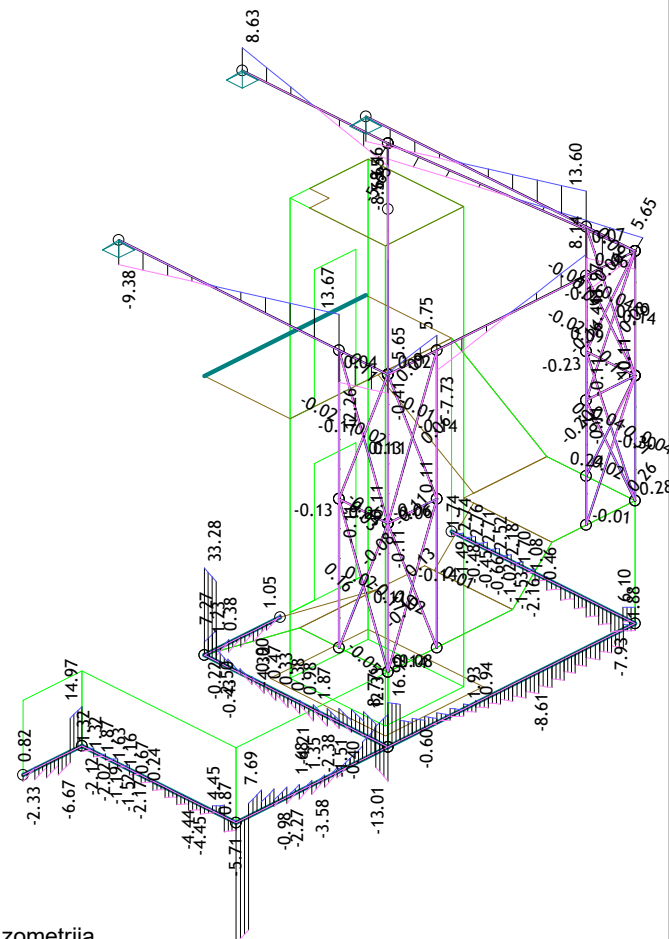
Obt. 313: [MSN] 12-199,208-222,239-243,252-254



Izometrija

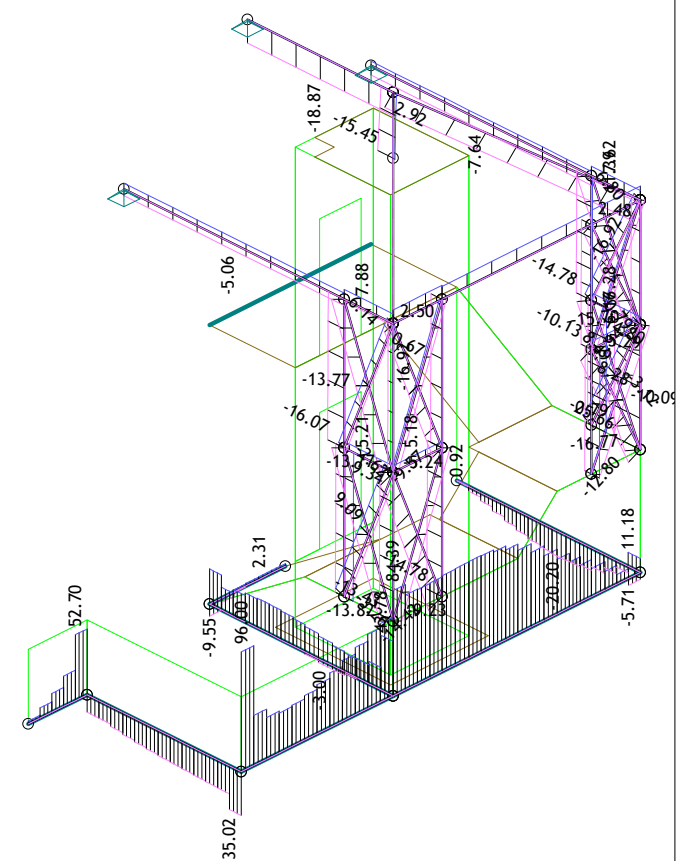
Vplivi v gredi: max T3= 57.81 / min T3= -68.01 kNm

Obt. 313: [MSN] 12-199,208-222,239-243,252-254



Izometrija

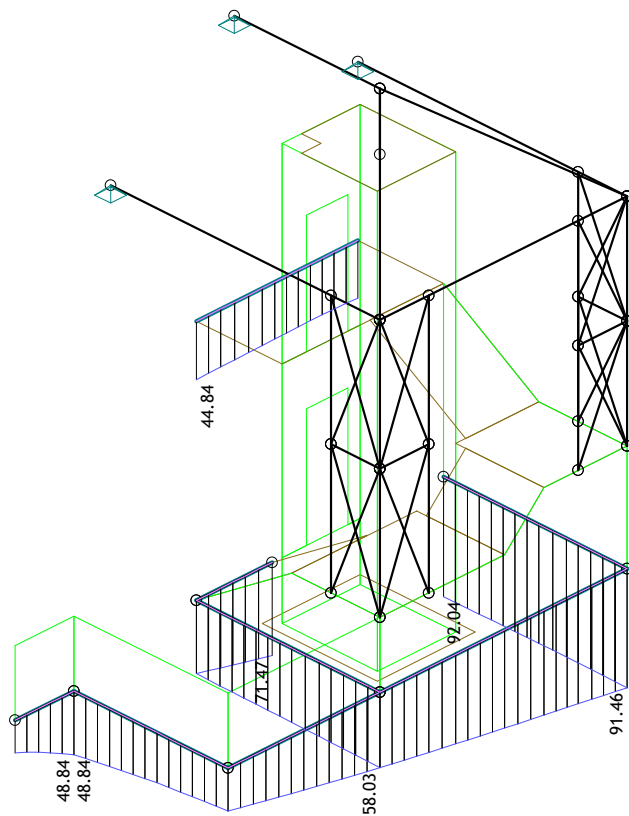
Vplivi v gredi: max T2= 33.28 / min T2= -45.64 kNm



Izometrija

Vplivi v gredi: max N1= 96.00 / min N1= -35.02 kN

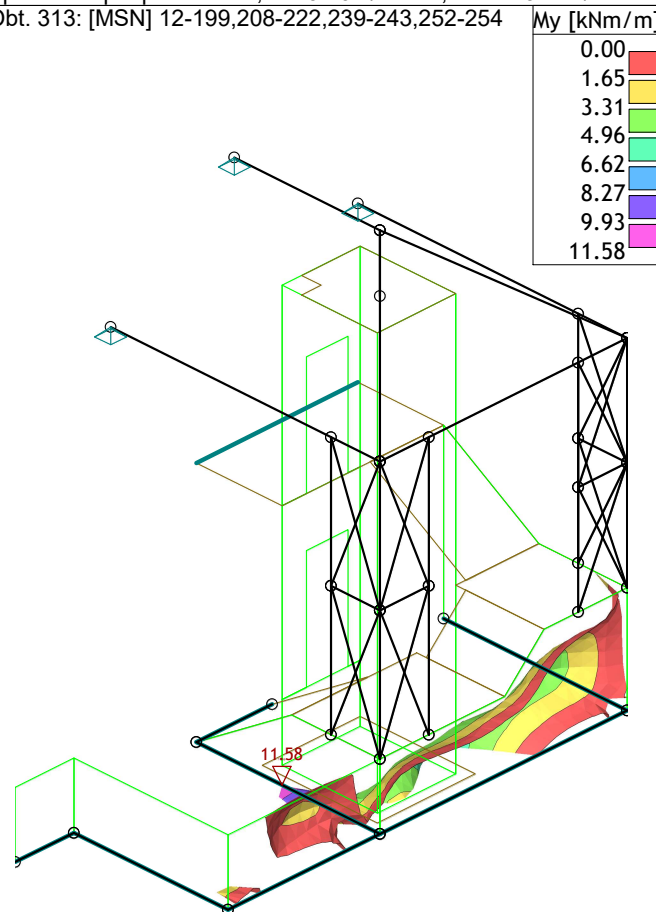
Obt. 313: [MSN] 12-199,208-222,239-243,252-254



Izometrija

Vplivi v lin. podpori: max σ_{tal} = 92.04 / min σ_{tal} = 14.31 kN/m²

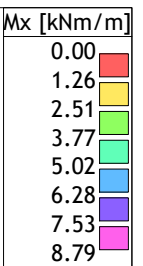
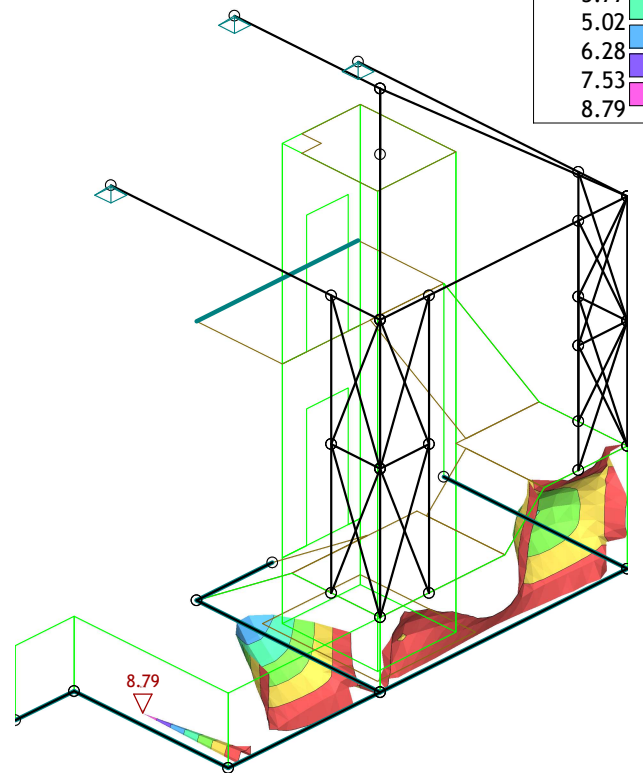
Obt. 313: [MSN] 12-199,208-222,239-243,252-254



Izometrija

Vplivi v plošči: max M_y = 42.67 / min M_y = 0.00 kNm/m

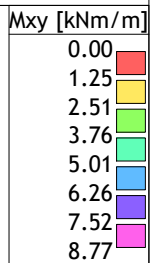
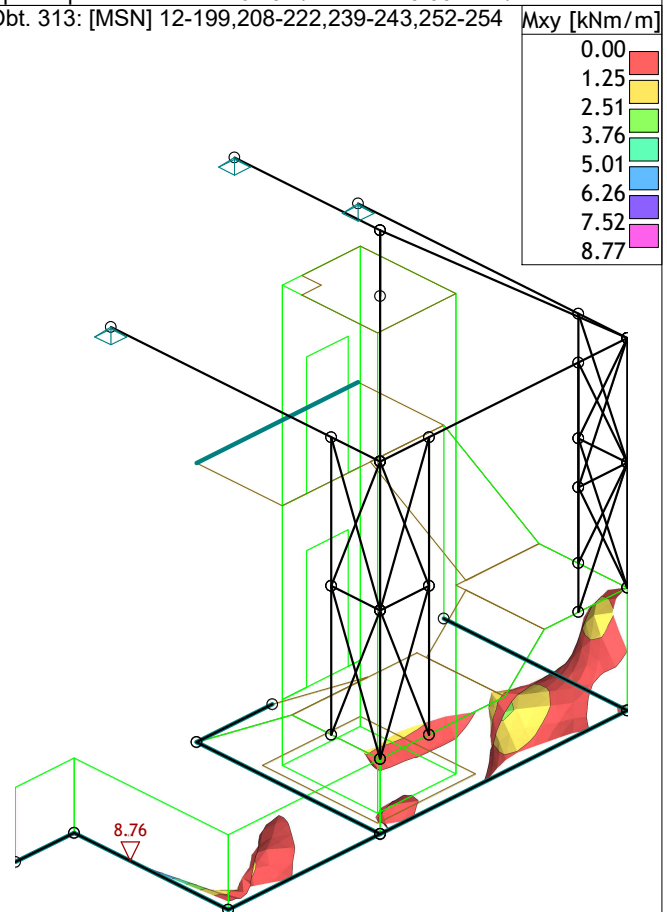
Obt. 313: [MSN] 12-199,208-222,239-243,252-254



Izometrija

Vplivi v plošči: max M_x = 64.94 / min M_x = 0.00 kNm/m

Obt. 313: [MSN] 12-199,208-222,239-243,252-254



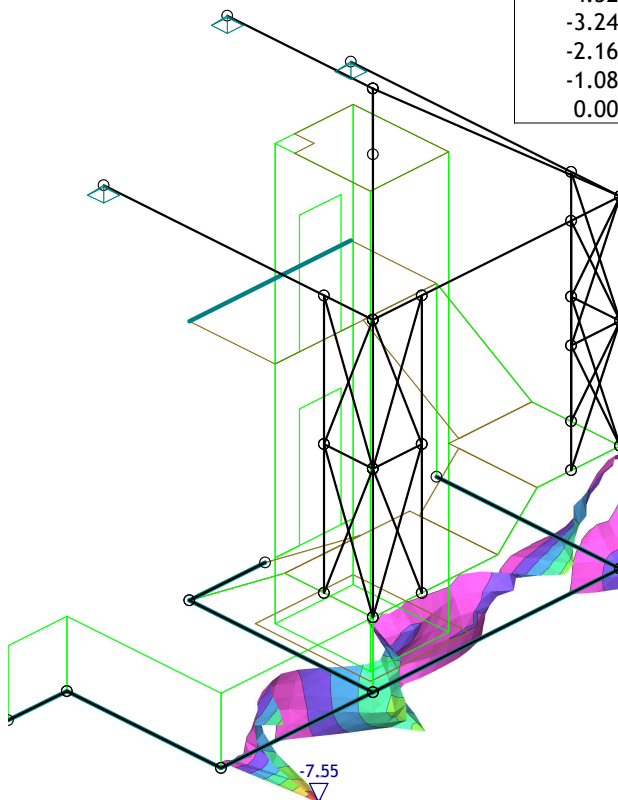
Izometrija

Vplivi v plošči: max M_{xy} = 9.47 / min M_{xy} = 0.00 kNm/m

Obt. 313: [MSN] 12-199,208-222,239-243,252-254

Mxy [kNm/m]

-7.56
-6.48
-5.40
-4.32
-3.24
-2.16
-1.08
0.00



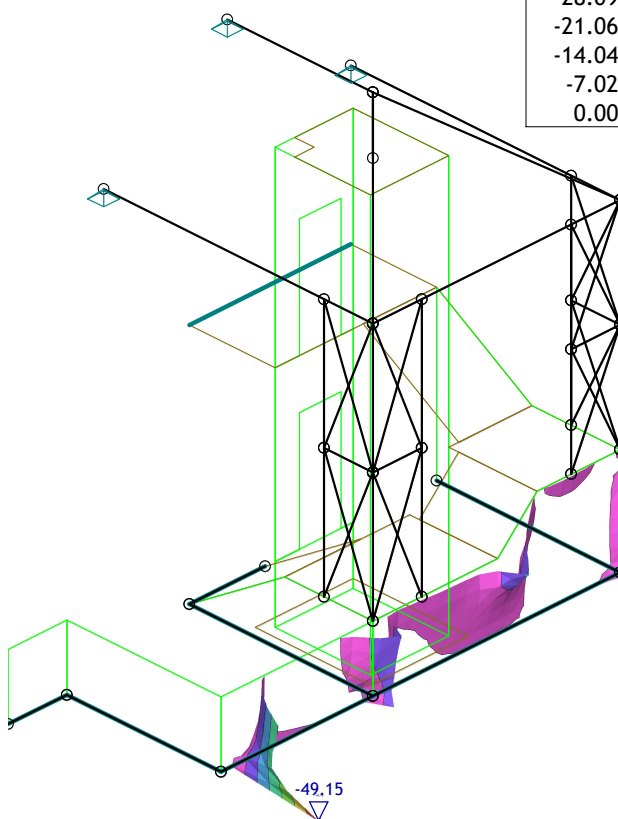
Izometrija

Vplivi v plošči: max Mxy= 0.00 / min Mxy= -7.55 kNm/m

Obt. 313: [MSN] 12-199,208-222,239-243,252-254

Mx [kNm/m]

-49.15
-42.13
-35.11
-28.09
-21.06
-14.04
-7.02
0.00



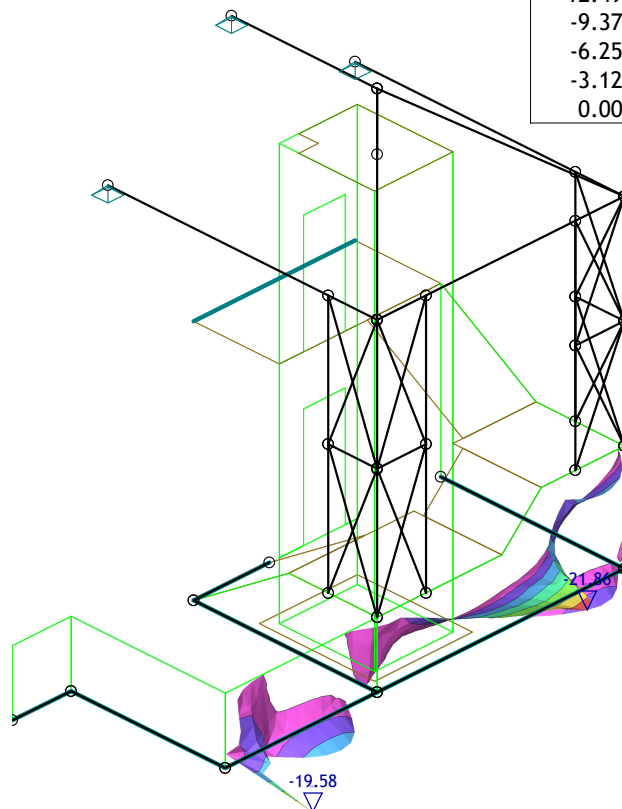
Izometrija

Vplivi v plošči: max Mx= 0.00 / min Mx= -49.15 kNm/m

Obt. 313: [MSN] 12-199,208-222,239-243,252-254

My [kNm/m]

-21.86
-18.74
-15.61
-12.49
-9.37
-6.25
-3.12
0.00



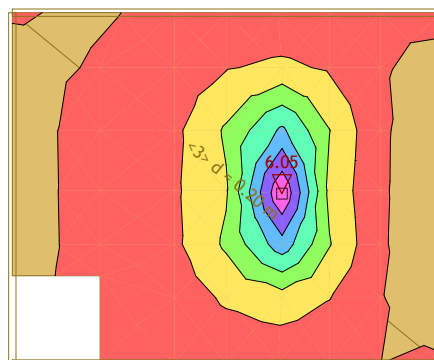
Izometrija

Vplivi v plošči: max My= 0.00 / min My= -21.86 kNm/m

Obt. 313: [MSN] 12-199,208-222,239-243,252-254

Mx [kNm/m]

0.00
0.86
1.73
2.59
3.46
4.32
5.19
6.05



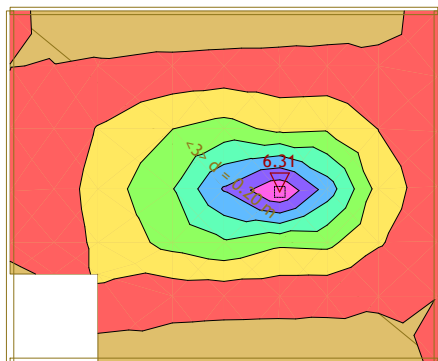
Nivo: [8.01 m]

Vplivi v plošči: max Mx= 6.05 / min Mx= 0.00 kNm/m

Obt. 313: [MSN] 12-199,208-222,239-243,252-254

My [kNm/m]

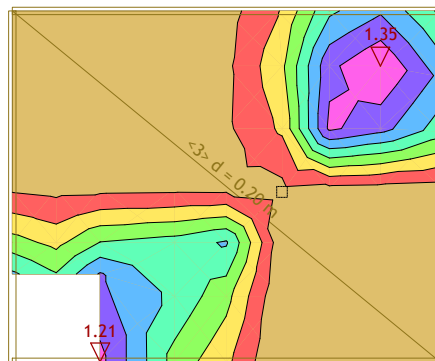
0.00
0.90
1.80
2.70
3.61
4.51
5.41
6.31



Obt. 313: [MSN] 12-199,208-222,239-243,252-254

Mxy [kNm/m]

0.00
0.19
0.39
0.58
0.77
0.96
1.16
1.35



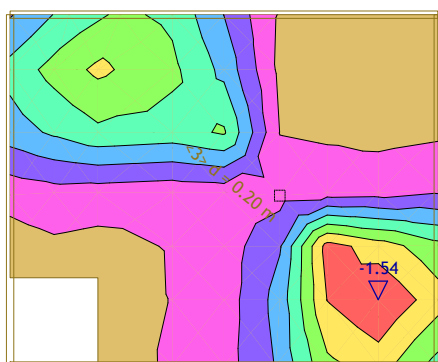
Nivo: [8.01 m]

Vplivi v plošči: max My= 6.31 / min My= 0.00 kNm/m

Obt. 313: [MSN] 12-199,208-222,239-243,252-254

Mxy [kNm/m]

-1.55
-1.33
-1.11
-0.89
-0.66
-0.44
-0.22
0.00



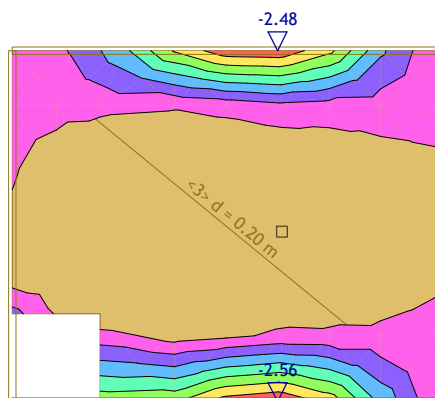
Nivo: [8.01 m]

Vplivi v plošči: max Mxy= 1.35 / min Mxy= 0.00 kNm/m

Obt. 313: [MSN] 12-199,208-222,239-243,252-254

My [kNm/m]

-2.56
-2.19
-1.83
-1.46
-1.10
-0.73
-0.37
0.00

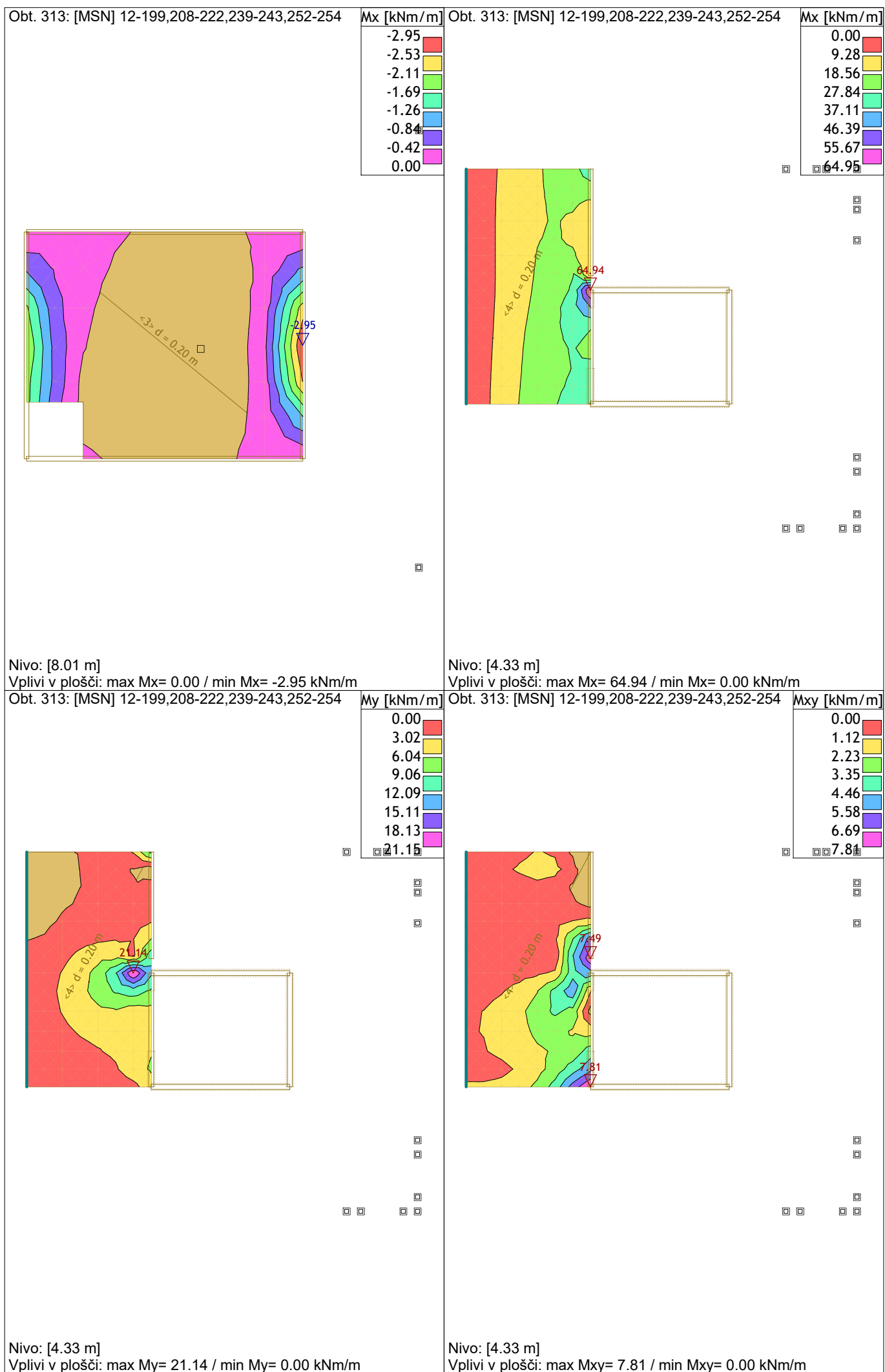


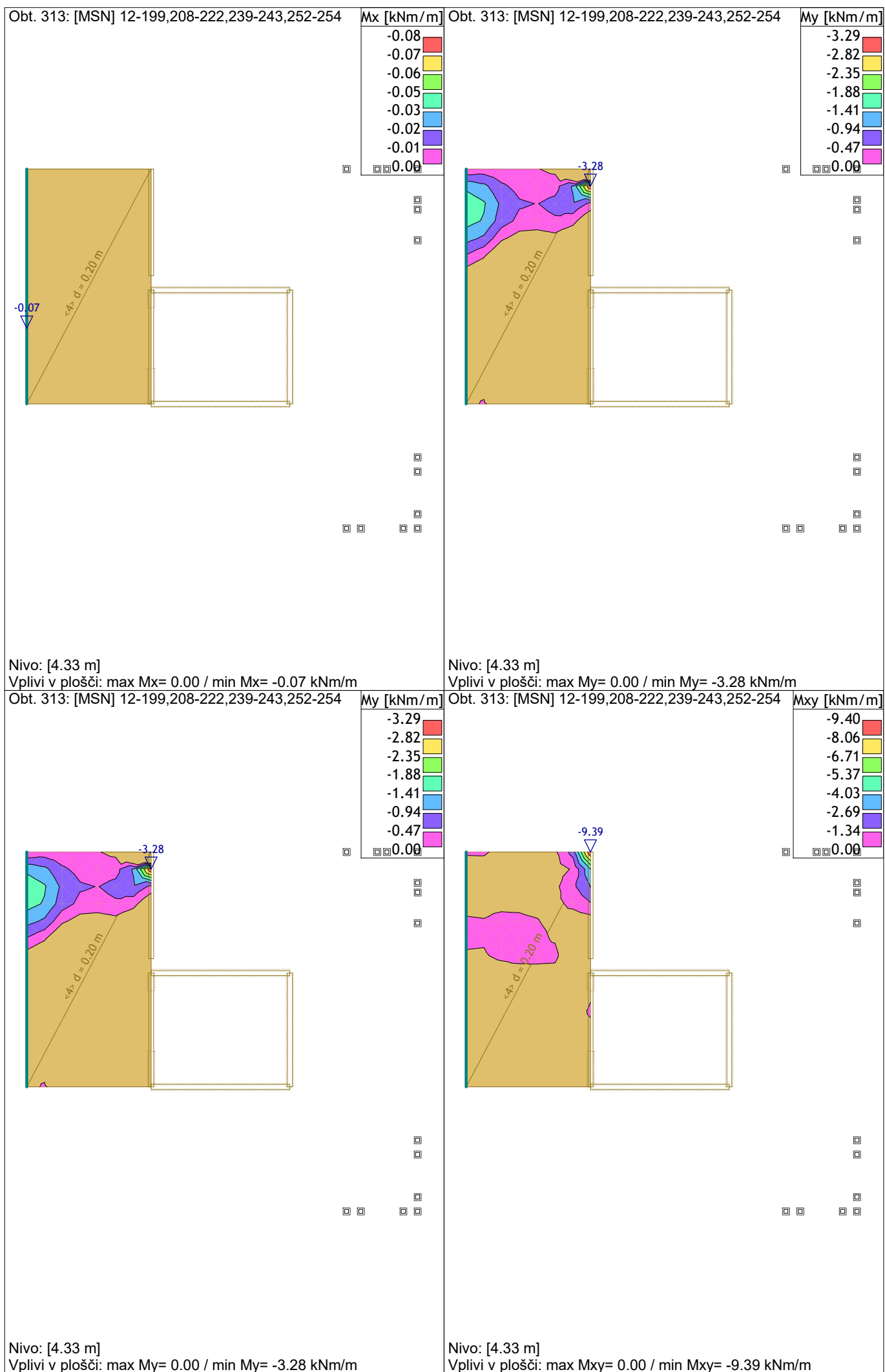
Nivo: [8.01 m]

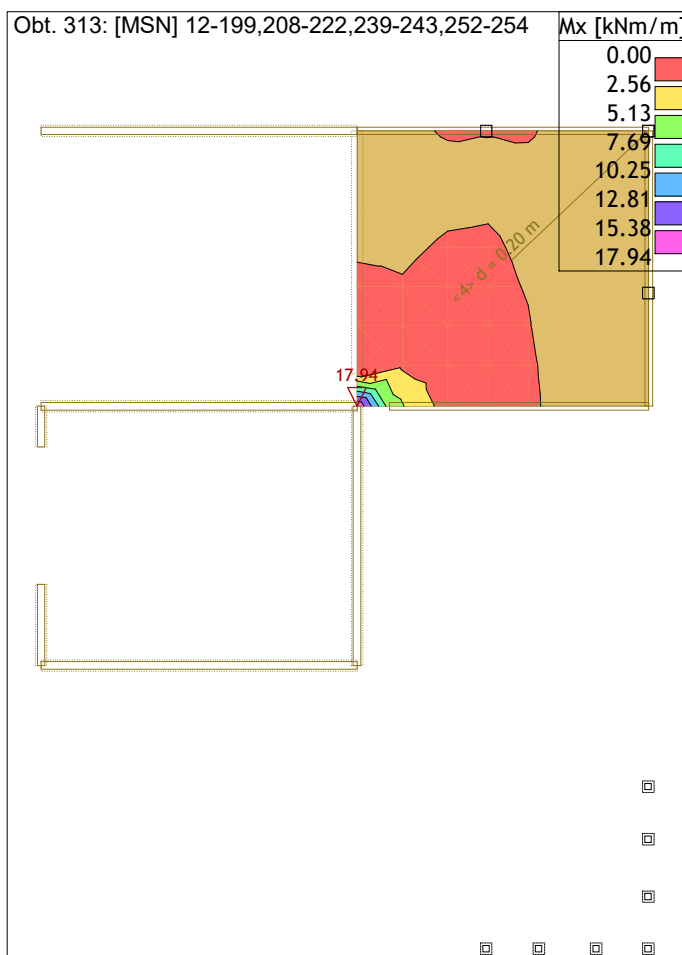
Vplivi v plošči: max Mxy= 0.00 / min Mxy= -1.54 kNm/m

Nivo: [8.01 m]

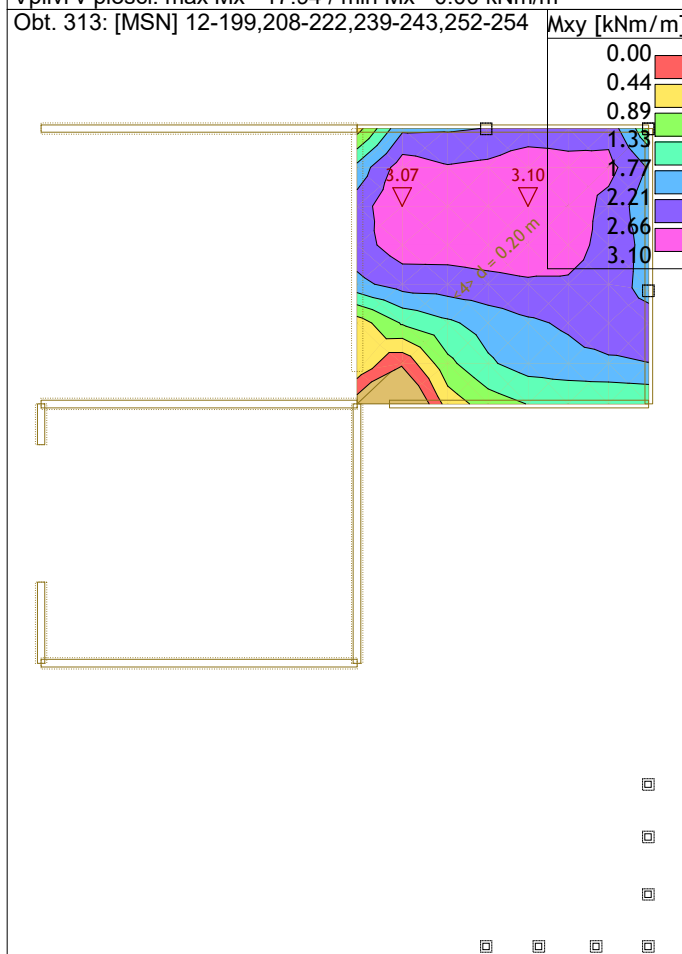
Vplivi v plošči: max My= 0.00 / min My= -2.56 kNm/m



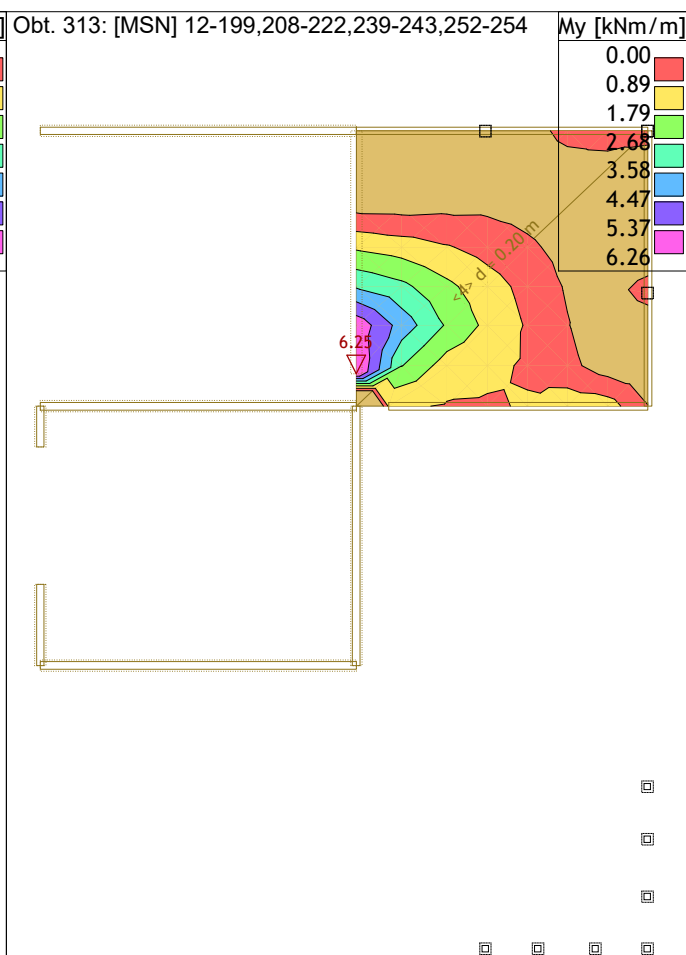




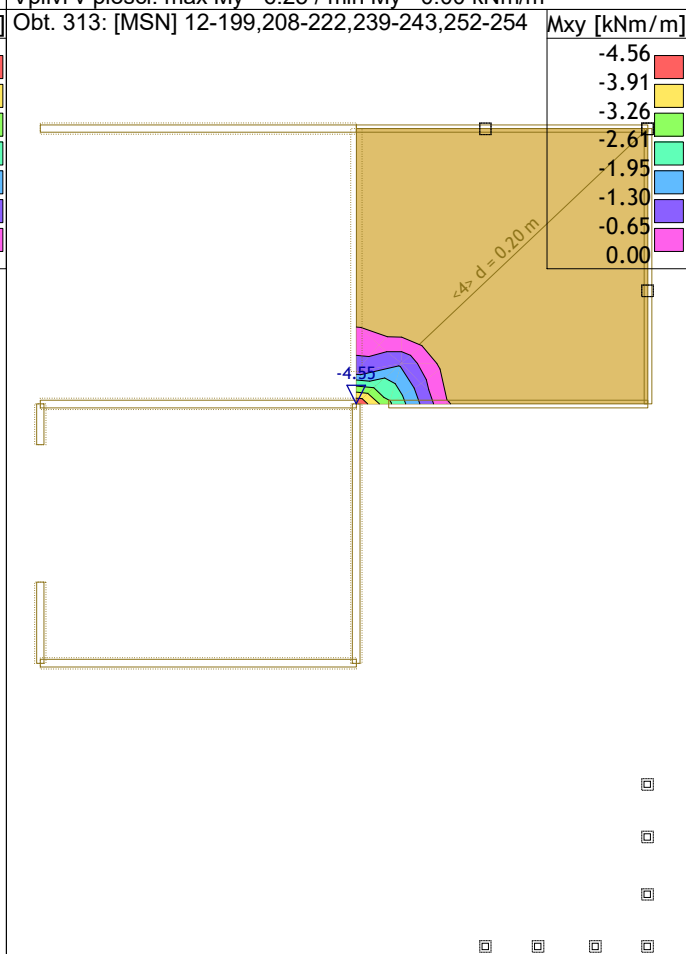
Nivo: [3.15 m]
Vplivi v plošči: max Mx= 17.94 / min Mx= 0.00 kNm/m



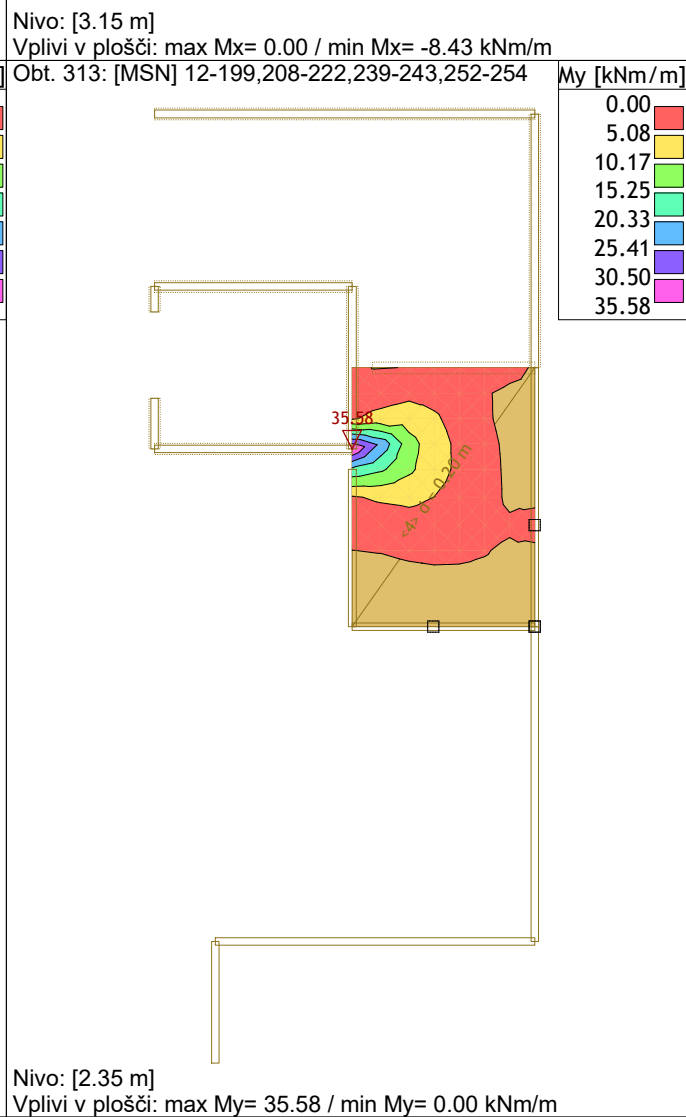
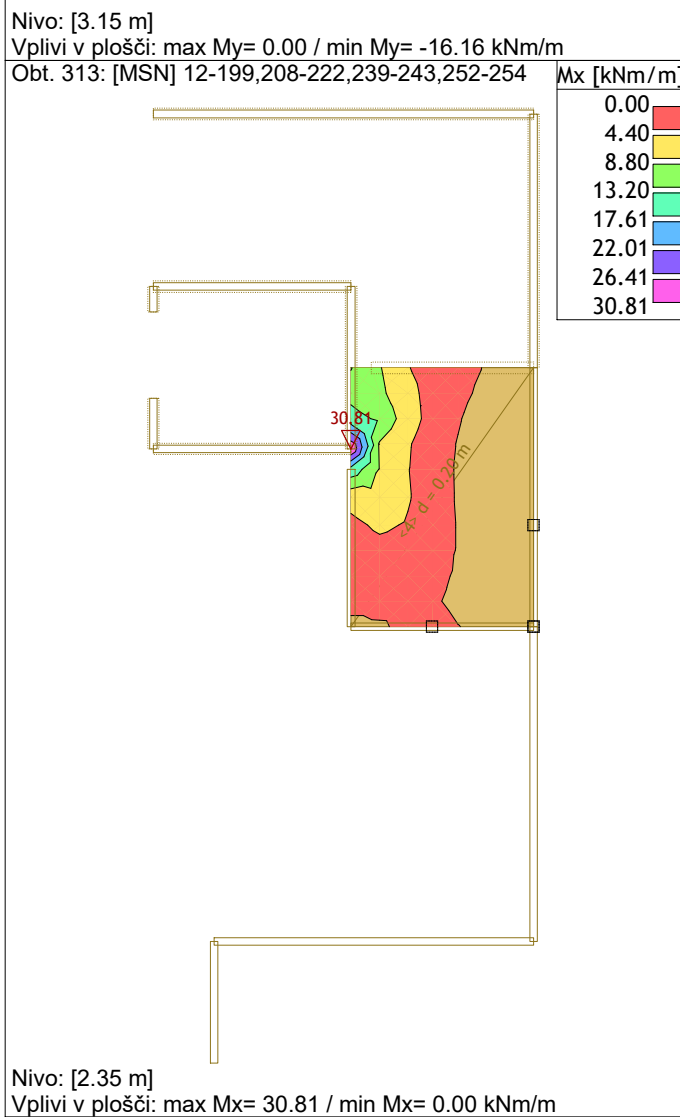
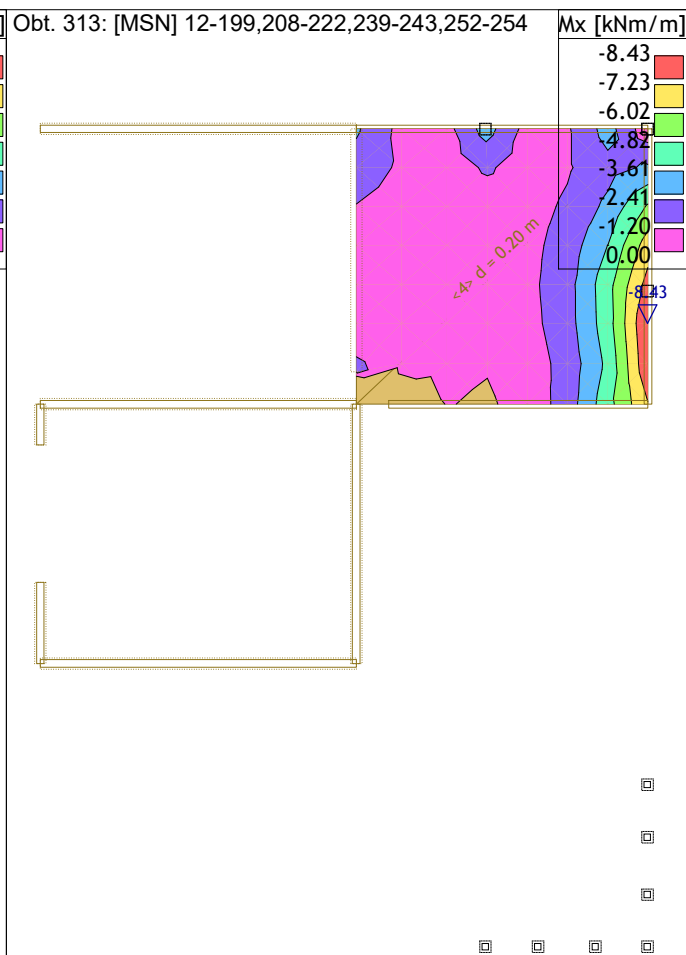
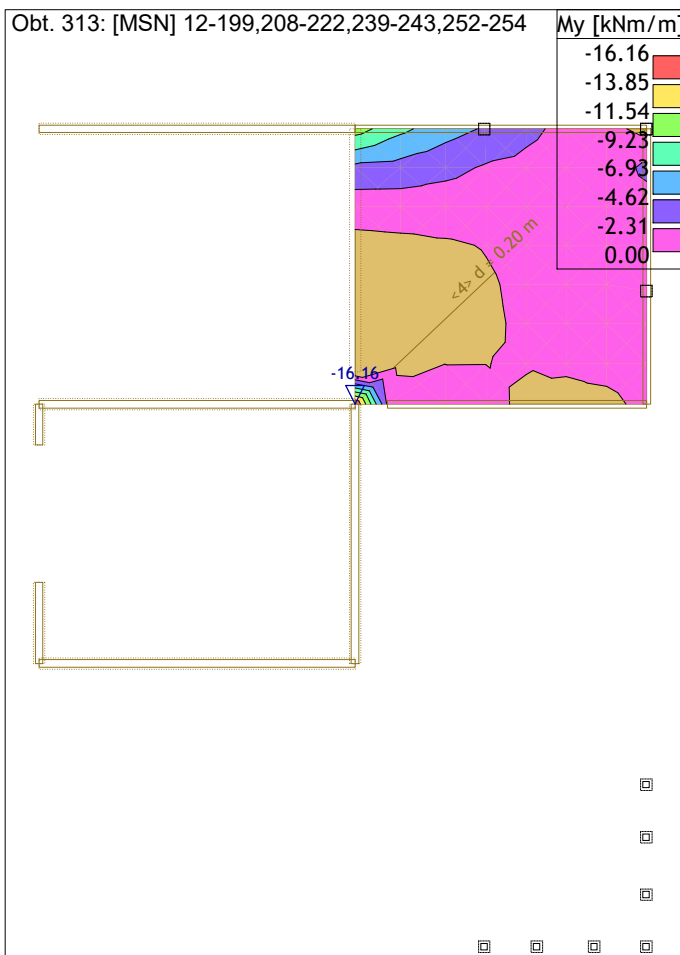
Nivo: [3.15 m]
Vplivi v plošči: max Mxy= 3.10 / min Mxy= 0.00 kNm/m



Nivo: [3.15 m]
Vplivi v plošči: max My= 6.25 / min My= 0.00 kNm/m



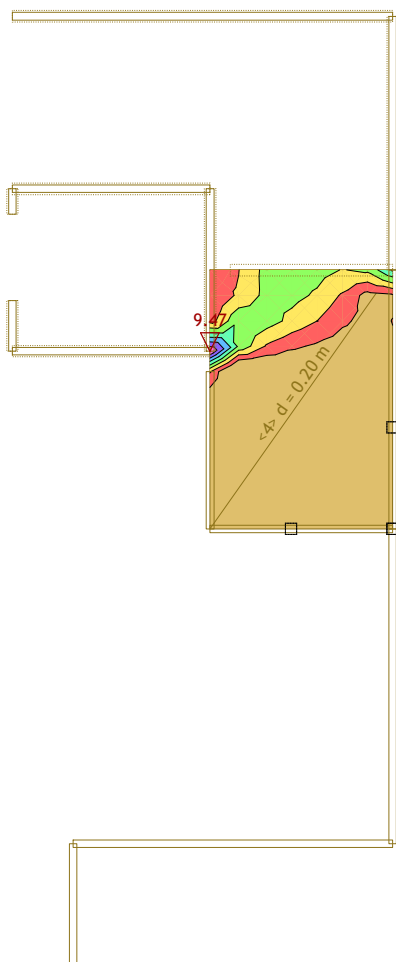
Nivo: [3.15 m]
Vplivi v plošči: max Mxy= 0.00 / min Mxy= -4.55 kNm/m



Obt. 313: [MSN] 12-199,208-222,239-243,252-254

Mxy [kNm/m]

0.00
1.35
2.71
4.06
5.42
6.77
8.13
9.48



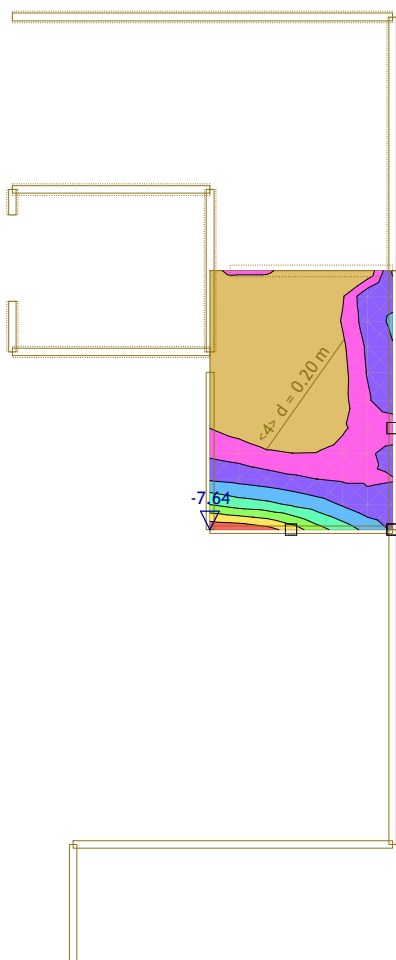
Nivo: [2.35 m]

Vplivi v plošči: max Mxy= 9.47 / min Mxy= 0.00 kNm/m

Obt. 313: [MSN] 12-199,208-222,239-243,252-254

My [kNm/m]

-7.65
-6.56
-5.46
-4.37
-3.28
-2.19
-1.09
0.00



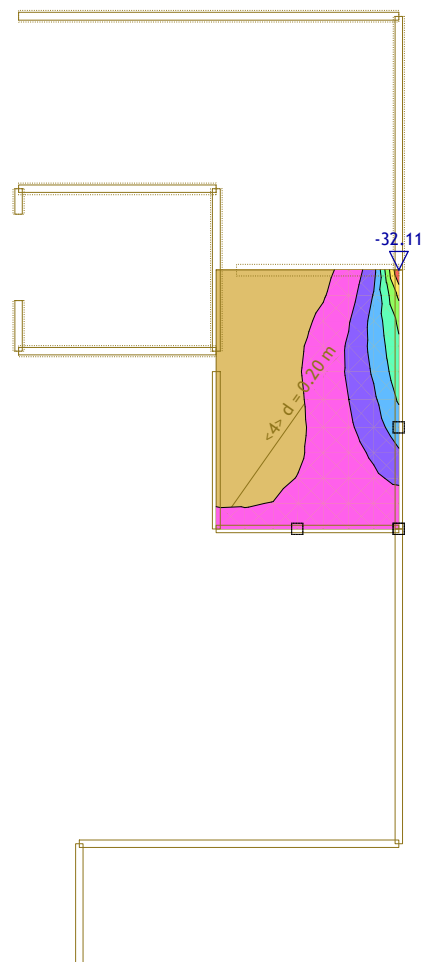
Nivo: [2.35 m]

Vplivi v plošči: max My= 0.00 / min My= -7.64 kNm/m

Obt. 313: [MSN] 12-199,208-222,239-243,252-254

Mx [kNm/m]

-32.12
-27.53
-22.94
-18.35
-13.77
-9.18
-4.59
0.00



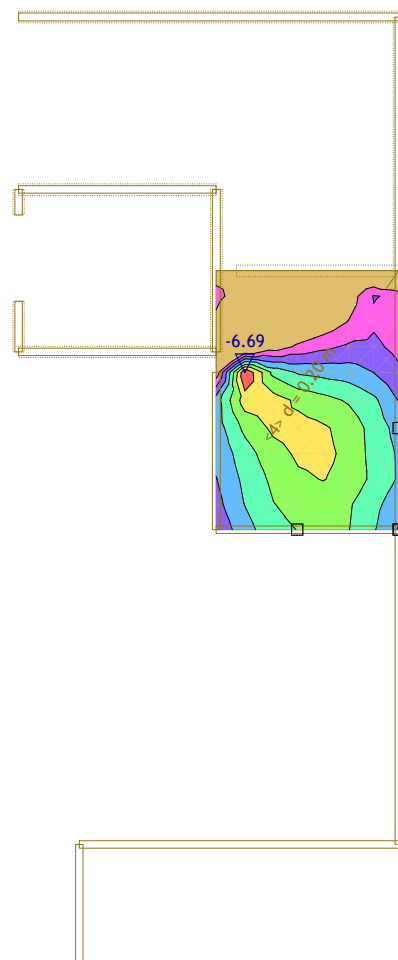
Nivo: [2.35 m]

Vplivi v plošči: max Mx= 0.00 / min Mx= -32.11 kNm/m

Obt. 313: [MSN] 12-199,208-222,239-243,252-254

Mxy [kNm/m]

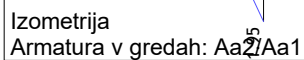
-6.69
-5.73
-4.78
-3.82
-2.87
-1.91
-0.96
0.00



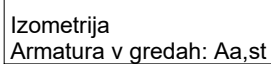
Nivo: [2.35 m]

Vplivi v plošči: max Mxy= 0.00 / min Mxy= -6.69 kNm/m

Merodajna obtežba: Kompletna shema
EC2 (EN 1992-1-1:2004), C25/30, B500B

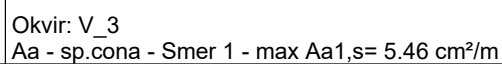


Merodajna obtežba: Kompletna shema
EC2 (EN 1992-1-1:2004), C25/30, B500B

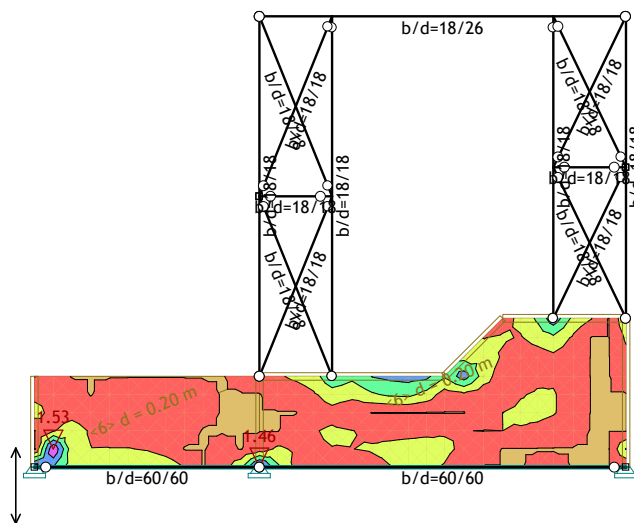


Izometrija
Armatura v gredah: Aa3/Aa4

Merodajna obtežba: Kompletna shema - sp.cona - Smer 1 [cm ² /m]	
EC2 (EN 1992-1-1:2004), C25/30, B500B, a=4.00 cm	0.00



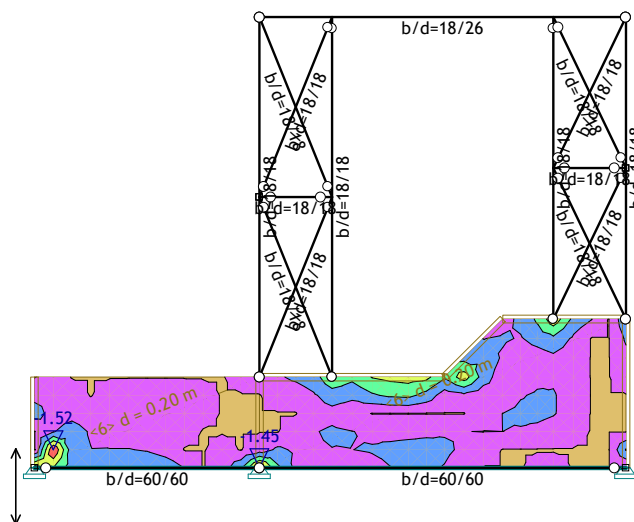
Merodajna optežba: Kompletna shema	Aa - sp.cona - Smer 2 [cm ² /m]
EC2 (EN 1992-1-1:2004), C25/30, B500B, a=4.00 cm	0.00
	0.31
	0.62
	0.92
	1.23
	1.54



Okvir: V_3

Aa - sp.cona - Smer 2 - max Aa2,s= 1.53 cm²/m

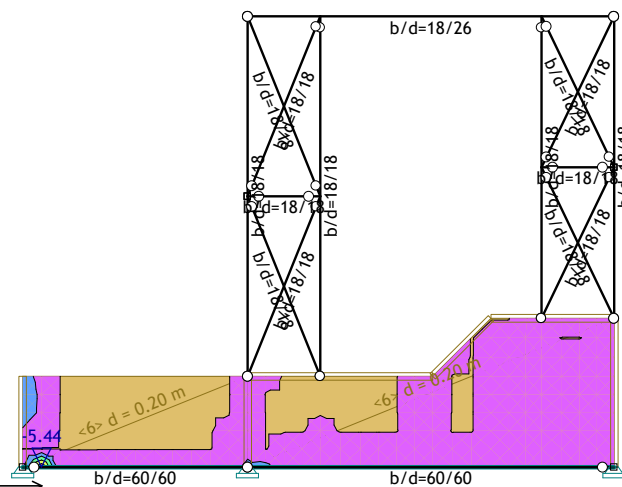
Merodajna optežba: Kompletna shema	Aa - zg.cona - Smer 2 [cm ² /m]
EC2 (EN 1992-1-1:2004), C25/30, B500B, a=4.00 cm	-1.53
	-1.22
	-0.92
	-0.61
	-0.31
	0.00



Okvir: V_3

Aa - zg.cona - Smer 2 - max Aa2,z= -1.52 cm²/m

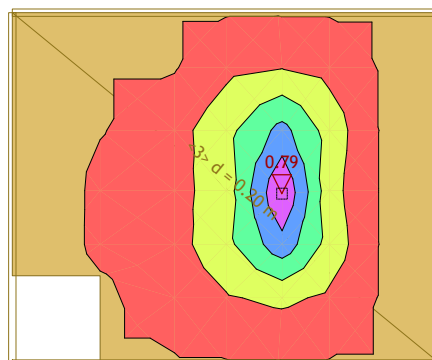
Merodajna optežba: Kompletna shema	Aa - zg.cona - Smer 1 [cm ² /m]
EC2 (EN 1992-1-1:2004), C25/30, B500B, a=4.00 cm	-5.44
	-4.35
	-3.26
	-2.18
	-1.09
	0.00



Okvir: V_3

Aa - zg.cona - Smer 1 - max Aa1,z= -5.44 cm²/m

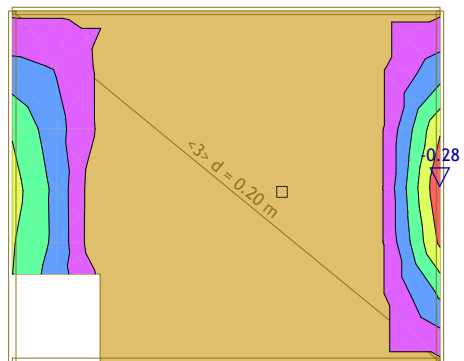
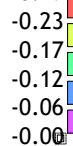
Merodajna optežba: Kompletna shema	Aa - sp.cona - Smer 1 [cm ² /m]
EC2 (EN 1992-1-1:2004), C25/30, B500B, a=4.00 cm	0.00
	0.16
	0.32
	0.47
	0.63
	0.79



Nivo: [8.01 m]

Aa - sp.cona - Smer 1 - max Aa1,s= 0.79 cm²/m

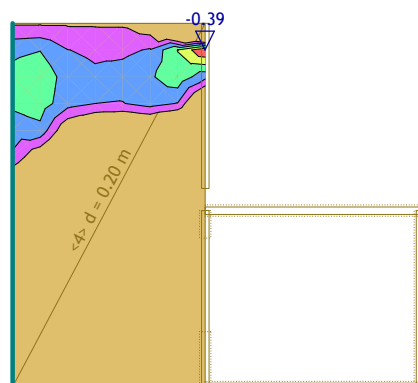
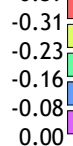
Am - zg.cona - Smer 1 [cm ² /m]	
B500B, a=4.00 cm	-0.29



Nivo: [8.01 m]

Aa - zg.cona - Smer 1 - max Aa1,z= -0.28 cm²/m

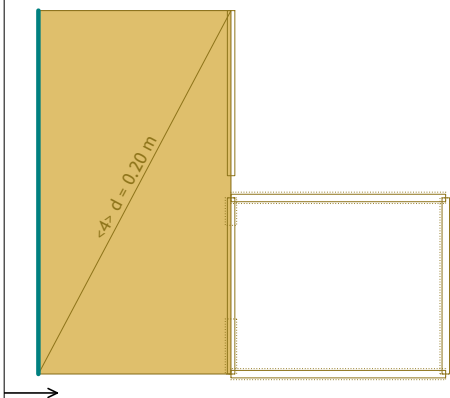
ana - zg.cona - Smer 2 [cm ² /m]	
B500B, a=4.00 cm	-0.39



Nivo: [4.33 m]

Aa - zg.cona - Smer 2 - max Aa2,z= -0.39 cm²/m

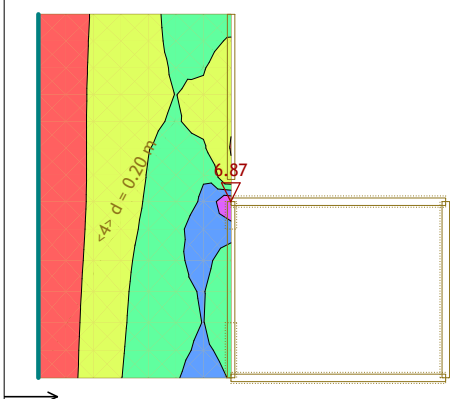
Merodajna obtežba: Kompletna shema
EC2 (EN 1992-1-1:2004), C25/30, B500B, a=4.00 cm



Nivo: [4.33 m]
Aa - zg.cona - Smer 1

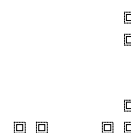
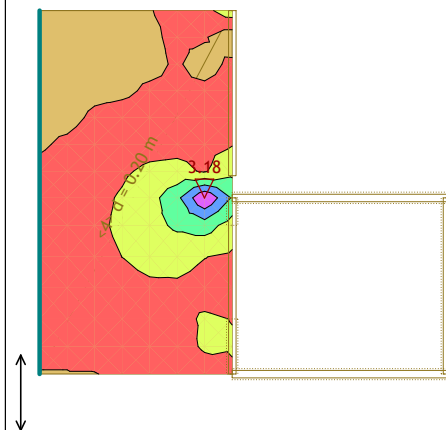
Merodajna obtežba: Kompletna shema
EC2 (EN 1992-1-1:2004), C25/30, B500B, a=4.00 cm

Aa - sp.cona - Smer 1 [cm²/m]
0.08
1.44
2.80
4.16
5.52
6.88



Nivo: [4.33 m]
Aa - sp.cona - Smer 1 - max Aa1,s= 6.87 cm²/m

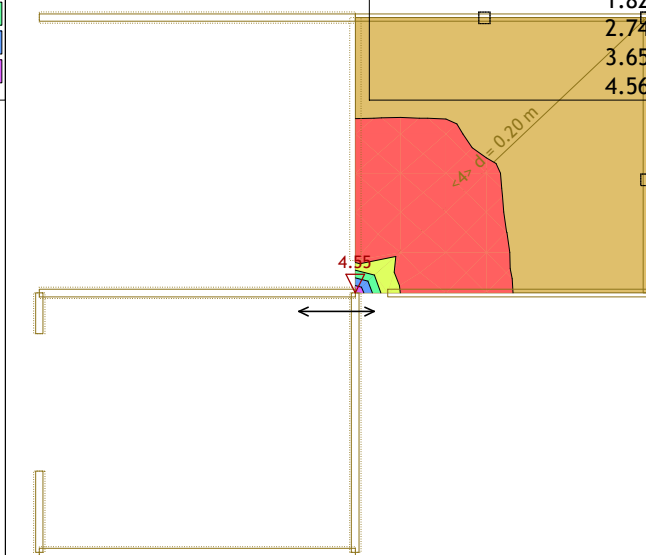
Merodajna obtežba: Kompletna shema
EC2 (EN 1992-1-1:2004), C25/30, B500B, a=4.00 cm



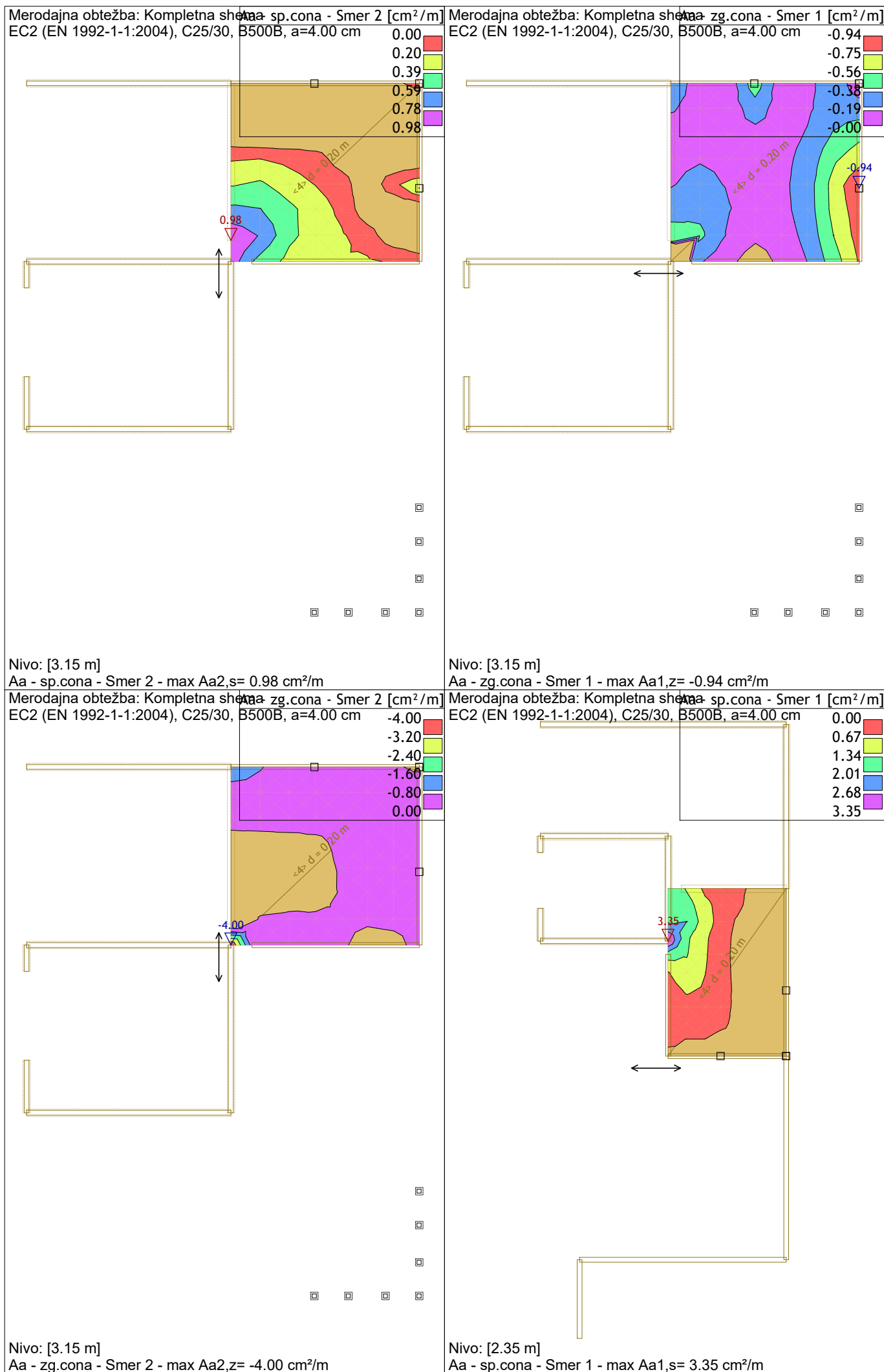
Nivo: [4.33 m]
Aa - sp.cona - Smer 2 - max Aa2,s= 3.18 cm²/m

Merodajna obtežba: Kompletna shema
EC2 (EN 1992-1-1:2004), C25/30, B500B, a=4.00 cm

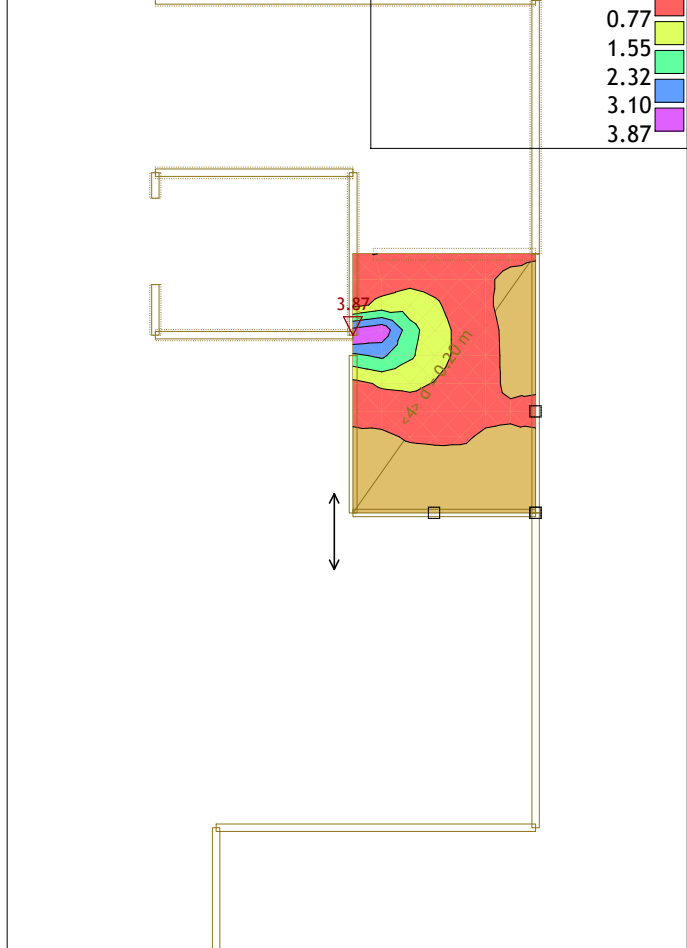
Aa - sp.cona - Smer 1 [cm²/m]
0.00
0.64
1.27
1.91
2.54
3.18



Nivo: [3.15 m]
Aa - sp.cona - Smer 1 - max Aa1,s= 4.55 cm²/m



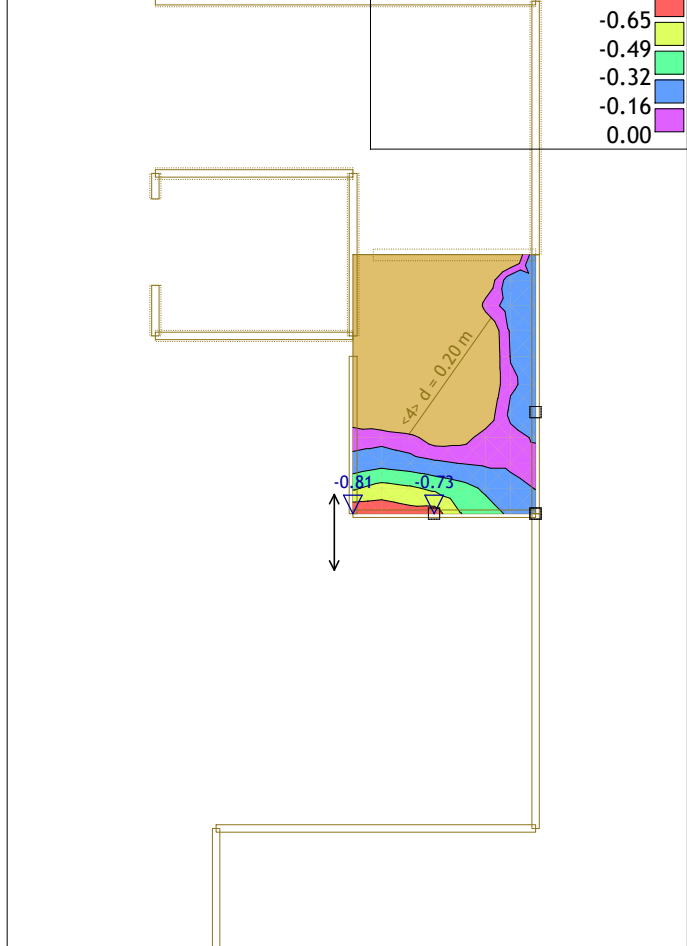
Merodajna obtežba: Kompletna shema
EC2 (EN 1992-1-1:2004), C25/30, B500B, a=4.00 cm



Nivo: [2.35 m]

Aa - sp.cona - Smer 2 - max Aa2,s= 3.87 cm²/m

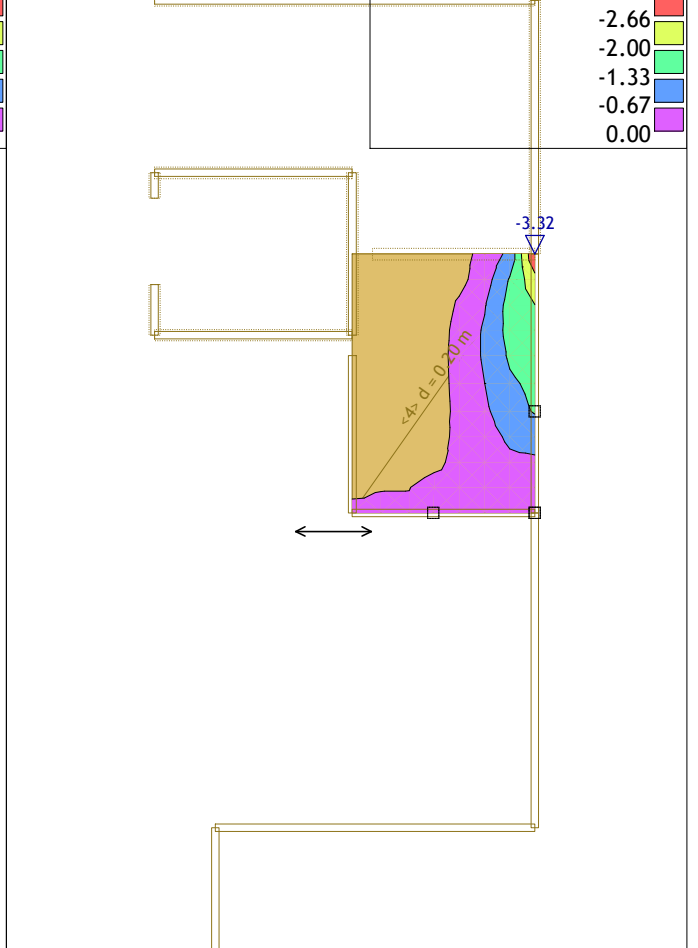
Merodajna obtežba: Kompletna shema
EC2 (EN 1992-1-1:2004), C25/30, B500B, a=4.00 cm



Nivo: [2.35 m]

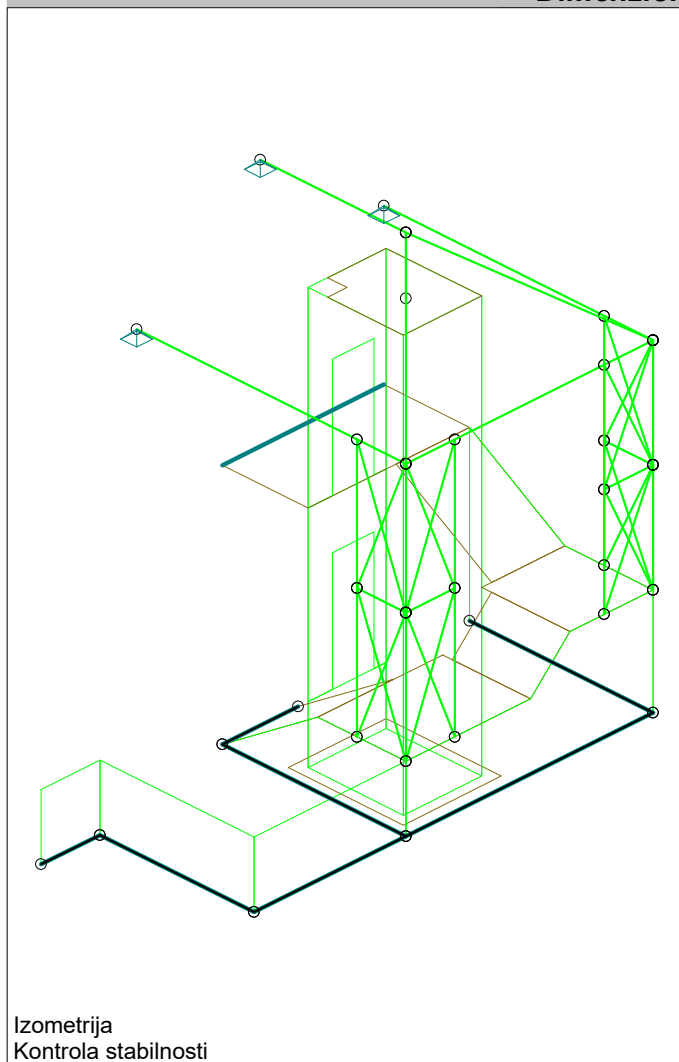
Aa - zg.cona - Smer 2 - max Aa2,z= -0.81 cm²/m

Merodajna obtežba: Kompletna shema
EC2 (EN 1992-1-1:2004), C25/30, B500B, a=4.00 cm



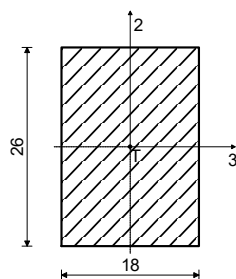
Nivo: [2.35 m]

Aa - zg.cona - Smer 1 - max Aa1,z= -3.32 cm²/m



PALICA 605-1769

Monoliten les - iglavci in mehki listavci - C24
Eksploatacijski razred 1
EUROCODE (EN 1995-1-1)



[cm]

FAKTORJI IZKORIŠČENOSTI PO KOMBINACIJAH OBTEŽB

310. $\gamma=0.32$	309. $\gamma=0.32$	42. $\gamma=0.30$
129. $\gamma=0.30$	159. $\gamma=0.30$	210. $\gamma=0.30$
49. $\gamma=0.30$	12. $\gamma=0.30$	126. $\gamma=0.30$
66. $\gamma=0.30$	156. $\gamma=0.30$	81. $\gamma=0.30$
22. $\gamma=0.30$	220. $\gamma=0.30$	47. $\gamma=0.30$
136. $\gamma=0.30$	79. $\gamma=0.30$	168. $\gamma=0.30$
50. $\gamma=0.29$	13. $\gamma=0.29$	137. $\gamma=0.29$
48. $\gamma=0.29$	23. $\gamma=0.29$	82. $\gamma=0.29$
80. $\gamma=0.29$	169. $\gamma=0.29$	151. $\gamma=0.27$
63. $\gamma=0.27$	73. $\gamma=0.27$	187. $\gamma=0.27$
101. $\gamma=0.27$	215. $\gamma=0.27$	20. $\gamma=0.27$
163. $\gamma=0.27$	148. $\gamma=0.27$	112. $\gamma=0.27$
36. $\gamma=0.27$	71. $\gamma=0.27$	241. $\gamma=0.27$
184. $\gamma=0.27$	162. $\gamma=0.27$	72. $\gamma=0.27$
74. $\gamma=0.27$	190. $\gamma=0.27$	21. $\gamma=0.26$
110. $\gamma=0.26$	191. $\gamma=0.26$	111. $\gamma=0.26$
113. $\gamma=0.26$	37. $\gamma=0.26$	60. $\gamma=0.22$
59. $\gamma=0.22$	145. $\gamma=0.22$	98. $\gamma=0.22$
181. $\gamma=0.22$	18. $\gamma=0.22$	69. $\gamma=0.22$
144. $\gamma=0.22$	97. $\gamma=0.22$	34. $\gamma=0.22$
16. $\gamma=0.22$	108. $\gamma=0.22$	57. $\gamma=0.22$
14. $\gamma=0.22$	180. $\gamma=0.22$	67. $\gamma=0.22$
32. $\gamma=0.22$	142. $\gamma=0.22$	143. $\gamma=0.22$
26. $\gamma=0.22$	95. $\gamma=0.22$	106. $\gamma=0.22$
55. $\gamma=0.22$	93. $\gamma=0.22$	56. $\gamma=0.22$
58. $\gamma=0.22$	178. $\gamma=0.22$	15. $\gamma=0.22$
247. $\gamma=0.21$	226. $\gamma=0.21$	70. $\gamma=0.21$
17. $\gamma=0.21$	234. $\gamma=0.21$	19. $\gamma=0.21$

35. $\gamma=0.21$	179. $\gamma=0.21$	266. $\gamma=0.21$
290. $\gamma=0.21$	203. $\gamma=0.21$	109. $\gamma=0.21$
256. $\gamma=0.21$	68. $\gamma=0.21$	269. $\gamma=0.21$
287. $\gamma=0.21$	96. $\gamma=0.21$	305. $\gamma=0.21$
33. $\gamma=0.21$	94. $\gamma=0.21$	293. $\gamma=0.21$
27. $\gamma=0.21$	107. $\gamma=0.21$	267. $\gamma=0.21$
257. $\gamma=0.21$	270. $\gamma=0.21$	268. $\gamma=0.21$
252. $\gamma=0.21$	294. $\gamma=0.21$	254. $\gamma=0.20$
227. $\gamma=0.20$	204. $\gamma=0.20$	248. $\gamma=0.20$
235. $\gamma=0.20$	236. $\gamma=0.20$	205. $\gamma=0.20$
228. $\gamma=0.19$	249. $\gamma=0.19$	312. $\gamma=0.19$
175. $\gamma=0.19$	90. $\gamma=0.19$	202. $\gamma=0.19$
88. $\gamma=0.19$	233. $\gamma=0.19$	25. $\gamma=0.19$
92. $\gamma=0.19$	177. $\gamma=0.19$	225. $\gamma=0.19$
246. $\gamma=0.19$	104. $\gamma=0.19$	30. $\gamma=0.19$
199. $\gamma=0.19$	91. $\gamma=0.19$	176. $\gamma=0.19$
123. $\gamma=0.19$	28. $\gamma=0.19$	122. $\gamma=0.19$
43. $\gamma=0.18$	134. $\gamma=0.18$	45. $\gamma=0.18$
197. $\gamma=0.18$	102. $\gamma=0.18$	24. $\gamma=0.18$
89. $\gamma=0.18$	119. $\gamma=0.18$	120. $\gamma=0.18$
174. $\gamma=0.18$	198. $\gamma=0.18$	121. $\gamma=0.18$
38. $\gamma=0.18$	87. $\gamma=0.18$	105. $\gamma=0.18$
39. $\gamma=0.18$	132. $\gamma=0.18$	311. $\gamma=0.18$
196. $\gamma=0.18$	118. $\gamma=0.18$	103. $\gamma=0.18$
29. $\gamma=0.18$	31. $\gamma=0.18$	46. $\gamma=0.18$
135. $\gamma=0.18$	44. $\gamma=0.18$	211. $\gamma=0.18$
133. $\gamma=0.18$	221. $\gamma=0.17$	302. $\gamma=0.16$
280. $\gamma=0.16$	262. $\gamma=0.16$	283. $\gamma=0.16$
212. $\gamma=0.16$	279. $\gamma=0.16$	131. $\gamma=0.16$
130. $\gamma=0.16$	277. $\gamma=0.16$	258. $\gamma=0.16$
260. $\gamma=0.16$	301. $\gamma=0.16$	253. $\gamma=0.16$
161. $\gamma=0.16$	222. $\gamma=0.16$	281. $\gamma=0.16$
299. $\gamma=0.16$	275. $\gamma=0.16$	160. $\gamma=0.15$
255. $\gamma=0.15$	308. $\gamma=0.15$	261. $\gamma=0.15$
284. $\gamma=0.15$	263. $\gamma=0.15$	300. $\gamma=0.15$
282. $\gamma=0.15$	278. $\gamma=0.15$	259. $\gamma=0.15$
276. $\gamma=0.15$	140. $\gamma=0.14$	53. $\gamma=0.14$
51. $\gamma=0.14$	127. $\gamma=0.14$	172. $\gamma=0.14$
83. $\gamma=0.14$	85. $\gamma=0.14$	138. $\gamma=0.14$
40. $\gamma=0.14$	157. $\gamma=0.14$	64. $\gamma=0.14$
208. $\gamma=0.14$	124. $\gamma=0.14$	209. $\gamma=0.13$
154. $\gamma=0.13$	218. $\gamma=0.13$	170. $\gamma=0.13$
128. $\gamma=0.13$	125. $\gamma=0.13$	41. $\gamma=0.13$
216. $\gamma=0.13$	52. $\gamma=0.13$	54. $\gamma=0.13$
306. $\gamma=0.13$	141. $\gamma=0.13$	173. $\gamma=0.13$
242. $\gamma=0.13$	86. $\gamma=0.13$	219. $\gamma=0.13$
139. $\gamma=0.13$	84. $\gamma=0.13$	158. $\gamma=0.13$
171. $\gamma=0.13$	200. $\gamma=0.13$	231. $\gamma=0.13$
155. $\gamma=0.13$	232. $\gamma=0.13$	223. $\gamma=0.13$
65. $\gamma=0.13$	244. $\gamma=0.13$	201. $\gamma=0.13$
224. $\gamma=0.13$	245. $\gamma=0.13$	206. $\gamma=0.12$
237. $\gamma=0.12$	229. $\gamma=0.12$	230. $\gamma=0.12$
207. $\gamma=0.12$	238. $\gamma=0.12$	250. $\gamma=0.12$
251. $\gamma=0.12$	217. $\gamma=0.12$	153. $\gamma=0.12$
152. $\gamma=0.12$	243. $\gamma=0.12$	189. $\gamma=0.12$
188. $\gamma=0.12$	307. $\gamma=0.12$	292. $\gamma=0.12$
291. $\gamma=0.12$	214. $\gamma=0.11$	150. $\gamma=0.11$
147. $\gamma=0.11$	62. $\gamma=0.10$	166. $\gamma=0.10$
75. $\gamma=0.10$	77. $\gamma=0.10$	194. $\gamma=0.10$
149. $\gamma=0.10$	61. $\gamma=0.10$	273. $\gamma=0.10$
116. $\gamma=0.10$	297. $\gamma=0.10$	146. $\gamma=0.10$
264. $\gamma=0.10$	288. $\gamma=0.10$	114. $\gamma=0.10$
99. $\gamma=0.10$	213. $\gamma=0.10$	164. $\gamma=0.10$
240. $\gamma=0.10$	295. $\gamma=0.10$	271. $\gamma=0.10$
185. $\gamma=0.10$	183. $\gamma=0.10$	239. $\gamma=0.10$
192. $\gamma=0.10$	303. $\gamma=0.10$	186. $\gamma=0.10$
285. $\gamma=0.10$	100. $\gamma=0.10$	182. $\gamma=0.10$
167. $\gamma=0.10$	298. $\gamma=0.10$	165. $\gamma=0.10$
76. $\gamma=0.10$	274. $\gamma=0.10$	78. $\gamma=0.10$
272. $\gamma=0.10$	304. $\gamma=0.10$	296. $\gamma=0.10$
195. $\gamma=0.10$	289. $\gamma=0.10$	286. $\gamma=0.10$
115. $\gamma=0.10$	117. $\gamma=0.10$	265. $\gamma=0.09$
193. $\gamma=0.09$		

KONTROLA NORMALNIH NAPETOSTI (obtežni primer 310, na 450.0 cm od začetka palice)

Računska osna sila	Ned = 11.214 kN
Prečna sila v smeri osi 2	V2ed = -12.797 kN
Prečna sila v smeri osi 3	V3ed ≈ 0.000 kN
Upogibni moment okoli osi 2	M2ed ≈ 0.000 kNm
Upogibni moment okoli osi 3	M3ed = 10.003 kNm

KONTROLA NAPETOSTI - NATEG IN UPOGIB

Vrsta obtežbe: osnovno - kratkotrajno	
Korekcijski koeficient	Kmod = 0.900
Parcialni koef. za karakteristike materiala	γ_m = 1.300
Dodatek za elemente z malimi dimenzijami - os 2	Kh_2 = 1.000
Dodatek za elemente z malimi dimenzijami - os 3	Kh_3 = 1.000
Dodatek za elemente z malimi dimenzijami - nateg	Kh_t = 1.000
Karakteristična natezna trdnost	ft,0,k = 14.000 MPa
Računska natezna trdnost	ft,0,d = 9.692 MPa
Faktor oblik (za pravokotni prerez)	km = 0.700
Karakteristična upogibna trdnost	fm,k = 24.000 MPa
Računska upogibna trdnost	fm,d = 16.615 MPa
Normalna natezna napetost	$\sigma_{t,0,d}$ = 0.240 MPa
Odpornostni moment	W3 = 2028.0 cm ³
Normalna upogibna napetost okoli osi 3	$\sigma_{m3,d}$ = 4.932 MPa

$$\sigma_{m3,d} \leq f_{m,d} \quad (4.932 \leq 16.615)$$

Izkoriščenost prereza je 29.7%

$$\sigma_{t,0,d} / f_{t,0,d} + k_m \times (\sigma_{m3,d} / f_{m,d}) + \sigma_{m2,d} / f_{m,d} \leq 1$$

$$(0.233 \leq 1)$$

Izkoriščenost prereza je 23.3%

$$\sigma_{t,0,d} / f_{t,0,d} + \sigma_{m3,d} / f_{m,d} + k_m \times (\sigma_{m2,d} / f_{m,d}) \leq 1$$

$$(0.322 \leq 1)$$

Izkoriščenost prereza je 32.2%

KONTROLA STRIŽNIH NAPETOSTI

(obtežni primer 309, na 450.0 cm od začetka palice)

Računska osna sila	Ned =	6.465 kN
Prečna sila v smeri osi 2	V2ed =	14.798 kN
Prečna sila v smeri osi 3	V3ed =	0.000 kN
Upogibni moment okoli osi 2	M2ed =	-0.116 kNm
Upogibni moment okoli osi 3	M3ed =	10.007 kNm

KONTROLA NAPETOSTI - STRIG

Vrsta obtežbe: osnovno - kratkotrajno

Korekcijski koeficient	Kmod =	0.900
Parcialni koef. za karakteristike materiala	γ_m =	1.300
Karakteristična strižna napetost	$f_{v,k}$ =	4.000 MPa
Računska strižna trdnost	$f_{v,d}$ =	2.769 MPa
Površina prečnega prereza	A =	468.00 cm ²
Dejanska strižna napetost(os 2)	$\tau_{2,d}$ =	0.474 MPa

$$\tau_{2,d} \leq f_{v,d} \quad (0.474 \leq 2.769)$$

Izkoriščenost prereza je 17.1%

DOKAZ BOČNE STABILNOSTI

Vrsta obtežbe: osnovno - kratkotrajno

Korekcijski koeficient	Kmod =	0.900
Parcialni koef. za karakteristike materiala	γ_m =	1.300
Razmak pridržanih točk pravokotno na smer osi 2	l_{ef} =	550.00 cm
5% fraktil modula E paralelno z vlakni	$E_{0.05}$ =	7400.0 MPa
5% fraktil strižnega modula G	$G_{0.05}$ =	460.00 MPa
Torzijski vztrajnostni moment	I_{tor} =	28693 cm ⁴
Vztrajnostni moment	I_2 =	12636 cm ⁴
Odpornostni moment	W_3 =	2028.0 cm ³
Kritična napetost uklona	$\sigma_{m,crit}$ =	98.948 MPa
Relativna vitkost za uklon	λ_{rel} =	0.492
Koeficient	k_{krit} =	1.000
Normalna upogibna napetost okoli osi 3	$\sigma_{m3,d}$ =	4.934 MPa

$$\sigma_{m3,d} \leq k_{krit} \times f_{m3,d} \quad (4.934 \leq 16.615)$$

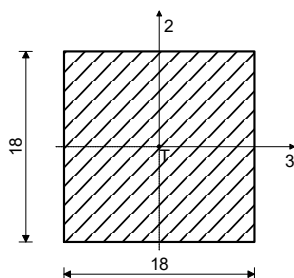
Izkoriščenost prereza je 29.7%

PALICA 1599-531

Monoliten les - iglavci in mehki listavci - C24

Eksplatacijski razred 1

EUROCODE (EN 1995-1-1)



[cm]

FAKTORJI IZKORIŠČENOSTI PO KOMBINACIJAH OBTEŽB

309. $\gamma=0.14$	310. $\gamma=0.14$	169. $\gamma=0.10$
220. $\gamma=0.10$	82. $\gamma=0.10$	159. $\gamma=0.10$
311. $\gamma=0.10$	168. $\gamma=0.10$	156. $\gamma=0.10$
80. $\gamma=0.10$	66. $\gamma=0.10$	23. $\gamma=0.10$
81. $\gamma=0.10$	79. $\gamma=0.10$	22. $\gamma=0.10$
137. $\gamma=0.10$	210. $\gamma=0.10$	50. $\gamma=0.09$
129. $\gamma=0.09$	48. $\gamma=0.09$	136. $\gamma=0.09$
126. $\gamma=0.09$	13. $\gamma=0.09$	49. $\gamma=0.09$
42. $\gamma=0.09$	47. $\gamma=0.09$	12. $\gamma=0.09$
312. $\gamma=0.09$	191. $\gamma=0.09$	241. $\gamma=0.09$
113. $\gamma=0.09$	187. $\gamma=0.09$	190. $\gamma=0.08$
184. $\gamma=0.08$	111. $\gamma=0.08$	112. $\gamma=0.08$
101. $\gamma=0.08$	37. $\gamma=0.08$	110. $\gamma=0.08$
163. $\gamma=0.08$	36. $\gamma=0.08$	215. $\gamma=0.08$
151. $\gamma=0.08$	74. $\gamma=0.08$	148. $\gamma=0.08$
72. $\gamma=0.08$	162. $\gamma=0.08$	21. $\gamma=0.08$
73. $\gamma=0.08$	63. $\gamma=0.07$	71. $\gamma=0.07$
20. $\gamma=0.07$	179. $\gamma=0.07$	96. $\gamma=0.07$
109. $\gamma=0.07$	181. $\gamma=0.07$	94. $\gamma=0.07$
178. $\gamma=0.07$	107. $\gamma=0.07$	180. $\gamma=0.07$

27. $\gamma=0.07$	33. $\gamma=0.07$	294. $\gamma=0.07$
305. $\gamma=0.07$	108. $\gamma=0.07$	97. $\gamma=0.07$
98. $\gamma=0.07$	35. $\gamma=0.07$	106. $\gamma=0.06$
95. $\gamma=0.06$	268. $\gamma=0.06$	270. $\gamma=0.06$
290. $\gamma=0.06$	293. $\gamma=0.06$	34. $\gamma=0.06$
269. $\gamma=0.06$	93. $\gamma=0.06$	287. $\gamma=0.06$
257. $\gamma=0.06$	266. $\gamma=0.06$	26. $\gamma=0.06$
143. $\gamma=0.06$	32. $\gamma=0.06$	267. $\gamma=0.06$
256. $\gamma=0.06$	58. $\gamma=0.06$	56. $\gamma=0.06$
70. $\gamma=0.06$	145. $\gamma=0.06$	144. $\gamma=0.06$
68. $\gamma=0.06$	142. $\gamma=0.06$	19. $\gamma=0.06$
15. $\gamma=0.06$	60. $\gamma=0.06$	69. $\gamma=0.06$
17. $\gamma=0.06$	59. $\gamma=0.06$	57. $\gamma=0.06$
67. $\gamma=0.06$	254. $\gamma=0.06$	55. $\gamma=0.06$
18. $\gamma=0.06$	16. $\gamma=0.06$	197. $\gamma=0.06$
14. $\gamma=0.05$	121. $\gamma=0.05$	135. $\gamma=0.05$
199. $\gamma=0.05$	119. $\gamma=0.05$	133. $\gamma=0.05$
198. $\gamma=0.05$	39. $\gamma=0.05$	134. $\gamma=0.05$
123. $\gamma=0.05$	46. $\gamma=0.05$	196. $\gamma=0.05$
122. $\gamma=0.05$	120. $\gamma=0.05$	44. $\gamma=0.05$
118. $\gamma=0.05$	132. $\gamma=0.05$	43. $\gamma=0.05$
45. $\gamma=0.05$	175. $\gamma=0.05$	90. $\gamma=0.05$
38. $\gamma=0.05$	177. $\gamma=0.04$	105. $\gamma=0.04$
252. $\gamma=0.04$	88. $\gamma=0.04$	300. $\gamma=0.04$
174. $\gamma=0.04$	104. $\gamma=0.04$	176. $\gamma=0.04$
278. $\gamma=0.04$	25. $\gamma=0.04$	221. $\gamma=0.04$
103. $\gamma=0.04$	91. $\gamma=0.04$	302. $\gamma=0.04$
284. $\gamma=0.04$	92. $\gamma=0.04$	31. $\gamma=0.04$
29. $\gamma=0.04$	276. $\gamma=0.04$	247. $\gamma=0.04$
299. $\gamma=0.04$	280. $\gamma=0.04$	301. $\gamma=0.04$
282. $\gamma=0.04$	283. $\gamma=0.04$	89. $\gamma=0.04$
259. $\gamma=0.04$	263. $\gamma=0.04$	102. $\gamma=0.04$
279. $\gamma=0.04$	222. $\gamma=0.04$	226. $\gamma=0.04$
234. $\gamma=0.04$	30. $\gamma=0.04$	246. $\gamma=0.04$
261. $\gamma=0.04$	277. $\gamma=0.04$	87. $\gamma=0.04$
203. $\gamma=0.04$	281. $\gamma=0.04$	260. $\gamma=0.04$
233. $\gamma=0.04$	275. $\gamma=0.04$	262. $\gamma=0.04$
28. $\gamma=0.04$	24. $\gamma=0.04$	225. $\gamma=0.04$
202. $\gamma=0.04$	244. $\gamma=0.04$	258. $\gamma=0.04$
219. $\gamma=0.04$	161. $\gamma=0.04$	231. $\gamma=0.04$
232. $\gamma=0.04$	245. $\gamma=0.04$	158. $\gamma=0.04$
200. $\gamma=0.04$	201. $\gamma=0.04$	223. $\gamma=0.04$
224. $\gamma=0.04$	160. $\gamma=0.04$	173. $\gamma=0.04$
155. $\gamma=0.04$	171. $\gamma=0.03$	211. $\gamma=0.03$
65. $\gamma=0.03$	308. $\gamma=0.03$	86. $\gamma=0.03$
172. $\gamma=0.03$	255. $\gamma=0.03$	218. $\gamma=0.03$
157. $\gamma=0.03$	216. $\gamma=0.03$	84. $\gamma=0.03$
170. $\gamma=0.03$	212. $\gamma=0.03$	253. $\gamma=0.03$
154. $\gamma=0.03$	83. $\gamma=0.03$	85. $\gamma=0.03$
209. $\gamma=0.03$	128. $\gamma=0.03$	64. $\gamma=0.03$
152. $\gamma=0.03$	153. $\gamma=0.03$	204. $\gamma=0.03$
75. $\gamma=0.03$	61. $\gamma=0.03$	130. $\gamma=0.03$
235. $\gamma=0.03$	131. $\gamma=0.03$	141. $\gamma=0.03$
227. $\gamma=0.03$	146. $\gamma=0.03$	77. $\gamma=0.03$
205. $\gamma=0.03$	248. $\gamma=0.03$	207. $\gamma=0.03$
125. $\gamma=0.03$	228. $\gamma=0.03$	164. $\gamma=0.03$
236. $\gamma=0.03$	249. $\gamma=0.03$	206. $\gamma=0.03$
139. $\gamma=0.03$	208. $\gamma=0.03$	41. $\gamma=0.03$
149. $\gamma=0.03$	230. $\gamma=0.03$	238. $\gamma=0.03$
166. $\gamma=0.03$	306. $\gamma=0.03$	52. $\gamma=0.03$
217. $\gamma=0.03$	140. $\gamma=0.03$	229. $\gamma=0.03$
54. $\gamma=0.03$	213. $\gamma=0.02$	76. $\gamma=0.02$
127. $\gamma=0.02$	237. $\gamma=0.02$	78. $\gamma=0.02$
138. $\gamma=0.02$	251. $\gamma=0.02$	307. $\gamma=0.02$
53. $\gamma=0.02$	250. $\gamma=0.02$	124. $\gamma=0.02$
242. $\gamma=0.02$	165. $\gamma=0.02$	62. $\gamma=0.02$
240. $\gamma=0.02$	40. $\gamma=0.02$	304. $\gamma=0.02$
51. $\gamma=0.02$	243. $\gamma=0.02$	186. $\gamma=0.02$
298. $\gamma=0.02$	291. $\gamma=0.02$	292. $\gamma=0.02$
289. $\gamma=0.02$	147. $\gamma=0.02$	195. $\gamma=0.02$
189. $\gamma=0.02$	286. $\gamma=0.02$	167. $\gamma=0.02$
297. $\gamma=0.02$	272. $\gamma=0.02$	274. $\gamma=0.02$
265. $\gamma=0.02$	183. $\gamma=0.02$	188. $\gamma=0.02$
303. $\gamma=0.02$	296. $\gamma=0.02$	99. $\gamma=0.02$
150. $\gamma=0.02$	288. $\gamma=0.02$	295. $\gamma=0.02$
239. $\gamma=0.02$	193. $\gamma=0.02$	100. $\gamma=0.02$
214. $\gamma=0.02$	194. $\gamma=0.02$	271. $\gamma=0.02$
115. $\gamma=0.02$	273. $\gamma=0.02$	116. $\gamma=0.02$
117. $\gamma=0.02$	114. $\gamma=0.02$	285. $\gamma=0.02$
192. $\gamma=0.02$	182. $\gamma=0.02$	185. $\gamma=0.02$
264. $\gamma=0.02$		

KONTROLA NORMALNIH NAPETOSTI (obtežni primer 309, konec palice)

Računska osna sila	Ned =	-21.678 kN
Prečna sila v smeri osi 2	V2ed ≈	0.000 kN

KONTROLA NAPETOSTI - TLAK

Vrsta obtežbe: osnovno - kratkotrajno
Korekcijski koeficient
Parcialni koef. za karakteristike materiala
Dodatek za elemente z malimi dimenzijami - os 2

Kmod =	0.900
γ_m =	1.300

Kh_2 = 1.000

Dodatek za elemente z malimi dimenzijami - os 3

Kh_3 =	1.000
km =	0.700
fc,0,k =	21.000 MPa
fc,0,d =	14.538 MPa
fm,k =	24.000 MPa
fm,d =	16.615 MPa
$\lambda_{rel,2}$ =	1.619
$\lambda_{rel,3}$ =	1.619

Faktor oblik (za pravokotni prerez)
Karakteristična tlačna trdnost
Računska tlačna trdnost
Karakteristična upogibna trdnost
Računska upogibna trdnost
Relativna vitkost
Relativna vitkost

Normalne tlačne napetosti	$\sigma_{c,0,d}$	=	0.669 MPa
TLAK IN UPOGIB - VELIKA VITKOST			
Začetna imperfekcija	β_c	=	0.200
Koeficient	k_3	=	1.942
Koeficient	k_2	=	1.942
Koeficient	$k_{c,3}$	=	0.332
Koeficient	$k_{c,2}$	=	0.332

$$(\sigma_{c,0,d} / (k_{c,2} \times f_{c,0,d})) + k_m \times (\sigma_{m3,d} / f_{m,d}) + \sigma_{m2,d} / f_{m,d} \leq 1 \quad (0.139 \leq 1)$$

Izkoriščenost prereza je 13.9%

$$(\sigma_{c,0,d} / (k_{c,3} \times f_{c,0,d})) + \sigma_{m3,d} / f_{m,d} + k_m \times (\sigma_{m2,d} / f_{m,d}) \leq 1 \quad (0.139 \leq 1)$$

Izkoriščenost prereza je 13.9%

KONTROLA STRIŽNIH NAPETOSTI (obtežni primer 203, začetek palice)

Prečna sila v smeri osi 2	V_{2ed}	≈	0.000 kN
Prečna sila v smeri osi 3	V_{3ed}	=	0.100 kN

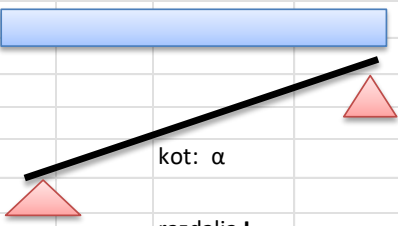
KONTROLA NAPETOSTI - STRIG

Vrsta obtežbe: osnovno - trenutno			
Korekcijski koeficient	K_{mod}	=	1.100
Parcialni koef. za karakteristike materiala	γ_m	=	1.300
Karakteristična strižna napetost	$f_{v,k}$	=	4.000 MPa
Računska strižna trdnost	$f_{v,d}$	=	3.385 MPa
Površina prečnega prereza	A	=	324.00 cm ²
Dejanska strižna napetost(os 3)	$\tau_{3,d}$	=	0.005 MPa

$$\tau_{3,d} \leq f_{v,d} \quad (0.005 \leq 3.385)$$

Izkoriščenost prereza je 0.1%

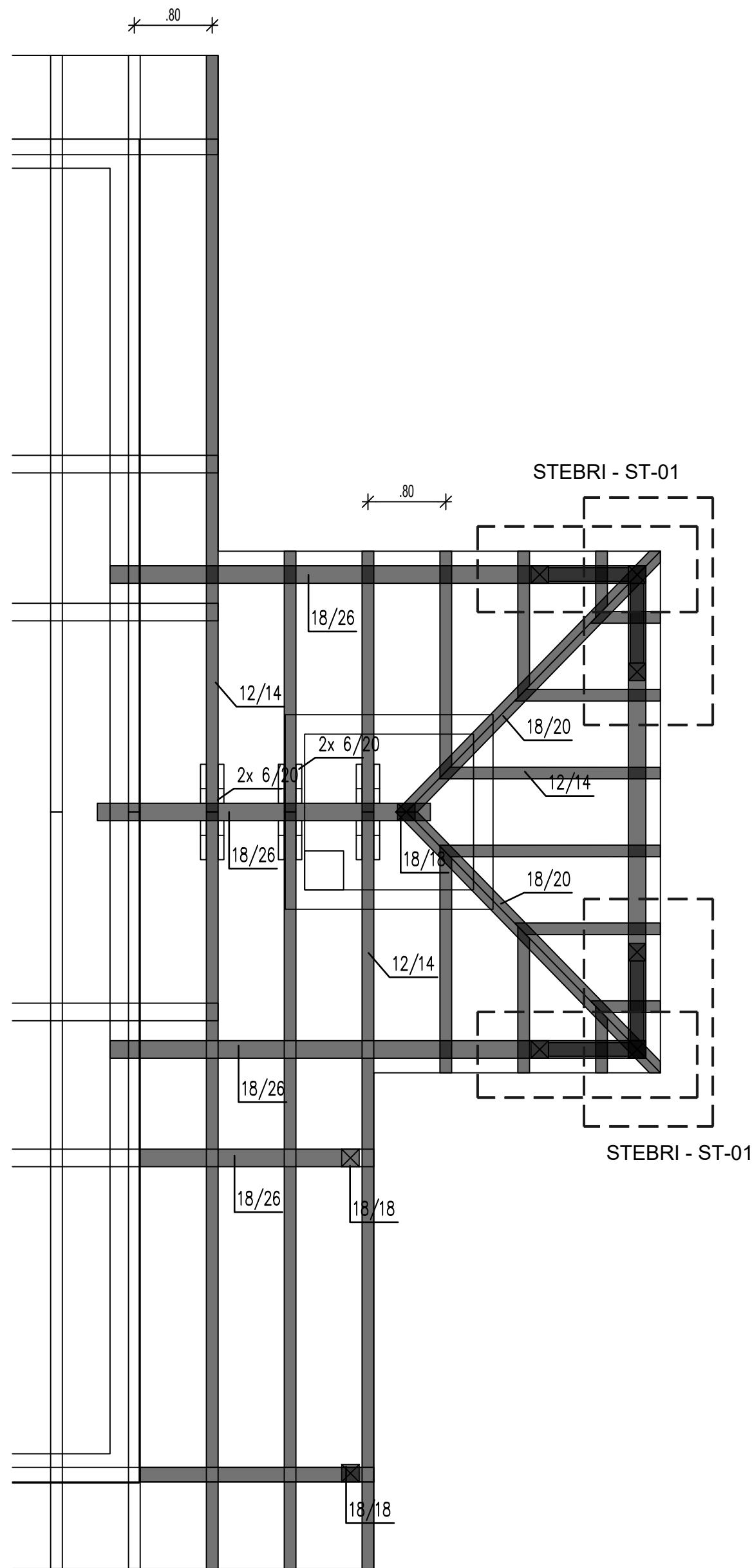
2.3.2.2. ANALIZA LESNIH KONSTRUKCIJA

Dimenzioniranje špirovec									
LES:	C 24			prerez					
PODATKI:	L =	2,65	m						
	$\alpha =$	30	°						
	e =	1,00	m						
DIMENZIJE:	b =	12	cm						
	h =	14	cm						
obtežba				$k_{cr} =$ 0,67 0,67..... za masiven les 1.... za ostale lesene proizvode					
				$\Delta h =$ 0,0 cm ...zasek					
razdalja L									
OBTEŽBA:	lastna	0,65	kN/m ²	→	0,65	kN/m ¹			
					0,08	kN/m ¹			
				g =	0,84	kN/m ¹			
	sneg	1,45	kN/m ²	→	1,45	kN/m ¹			
				s =	1,45	kN/m ¹			
	veter:	0,3	kN/m ²	→	0,3	kN/m ¹			
				w =	0,60	kN/m ¹			
OBREMENITEV:									
P	$M_{Ed,g} =$	1,000	kN/m	$V_{Ed,g} =$	1,509	kN	$k_{mod,1} =$	0,6	
M	$M_{Ed,g+s} =$	2,909	kN/m	$V_{Ed,g+s} =$	4,391	kN	$k_{mod,1} =$	0,8	
S	$M_{Ed,g+s+w} =$	3,383	kN/m	$V_{Ed,g+s+w} =$	5,106	kN	$k_{mod,1} =$	0,9	
DIMENZIONIRANJE:									
UPOGIB:					STRIG:				
	$\sigma_{m,y,d}$ [kN/cm ²]	<	$f_{m,d}$ [kN/cm ²]		τ_d [kN/cm ²]	<	$f_{v,d}$ [kN/cm ²]		
P	0,255	<	1,108	DA	0,020	<	0,115	DA	
M	0,742	<	1,477	DA	0,059	<	0,154	DA	
S	0,863	<	1,662	DA	0,068	<	0,173	DA	
KONTROLA BOČNE STABILNOSTI									
	$\sigma_{m,crit} =$	69,846		$\lambda_{rel,m} =$	0,185	<	0,75	DA	
POMIKI MSU									
$M_g =$	0,74	kN/m	TRENUTNA ZAČETNA DEFOMACIJA: L/250						
$M_s =$	1,27	kN/m	$v_{inst} =$	0,565	cm <	1,06	cm	DA	
$M_w =$	0,53	kN/m							
KONČNA DEFOMACIJA: L/200									
$v_{g,inst} =$	0,16	cm	$w_{fin} =$	0,696	cm <	1,33	cm	DA	
$v_{s,inst} =$	0,28	cm							
$v_{w,inst} =$	0,12	cm	DIMENZIJE ŠPIROVCA						
			b =	12	h =	14			

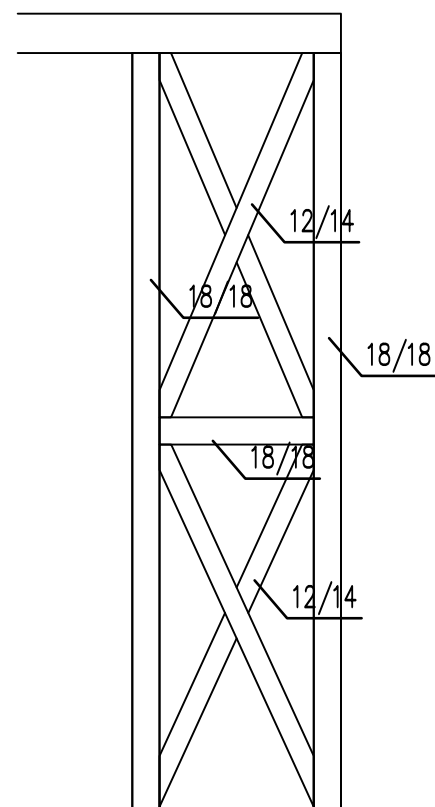
2. NAČRT GRADBENIŠTA

OSTALI LESOVI PREVERJENI V PRILOGI

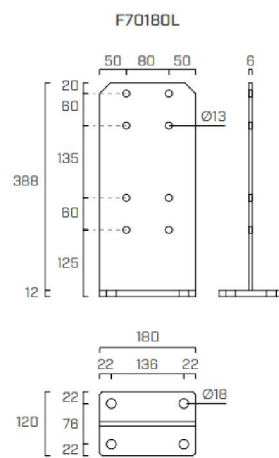
2.4. GRAFIČNE PODLOGE – LESENA KONSTRUKCIJA



TLORIS OSTREŠJA

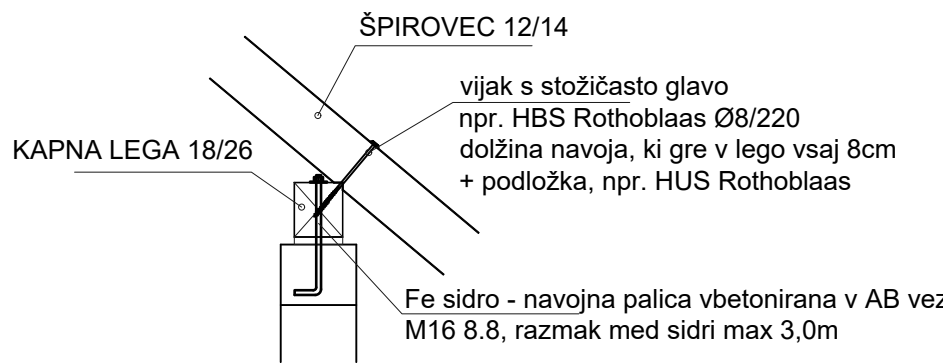


POGLED NA STEBRE - ST-01

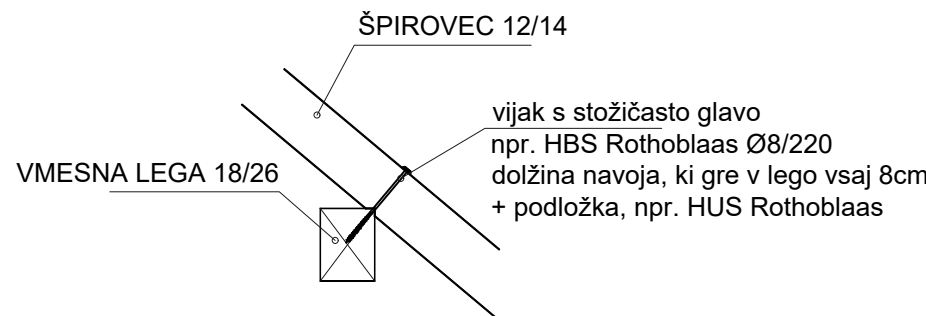


princip pritrdjevanja stebrov v ab beton

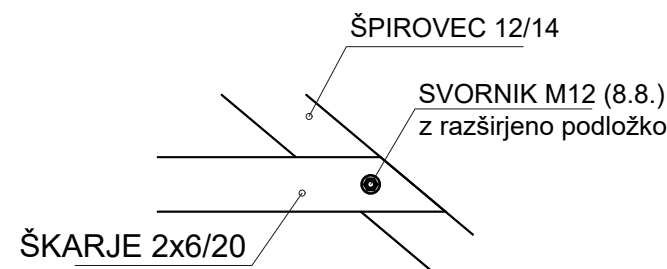
DETAJL SPOJA KAPNA LEGA-AB VEZ/PLOŠČA



DETAJL SPOJA VMESNA LEGA-ŠPIROVEC



DETAJL SPOJA ŠKARJE-ŠPIROVEC



MATERIALI:
- LES: C24
- JEKLO: S-235

OPOMBE:
- Poleg tega načrta potrebno upoštevati tudi ostale načrte kot npr. načrt inštalacij
- POSAMEZNE POZICIJE POTREBNO PRILAGODITI (pokriviti, porezati,...)
- VSE DIMEZIJE PREVERITI IN JIH USKLADITI NA LICU MESTA

Lesena konstrukcija

ZASNOVA
GRADBENI BIRO
Lovšin Tone s.p.
Vrvarska pot 1a
1310 Ribnica, Slovenija

LESENA KONSTRUKCIJA: OSTREŠJE

M 1:50 / 1:25

sestavni del:
objekt:
vodja projekta:
poblaščen inž./arh.:
sodelavec:

2. Načrt gradbeništva
PRENOVA ZDRAVSTVENE POSTAJE VELIKE LAŠČE
Miha Novina, univ. dipl. inž. arh., ZAPS 1654 A
Tone Lovšin, dipl. inž. grad., IZS G - 0379
Sandi Mohar, dipl. inž. grad., IZS G - 4126

št. projekta:
št. načrta:
faza projekta:
datum:
št. lista:

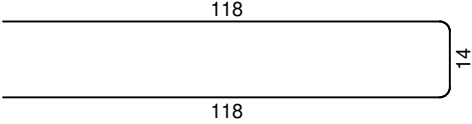
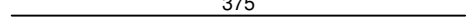
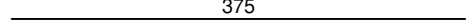
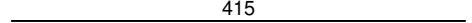
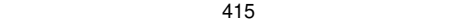
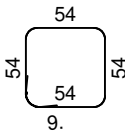
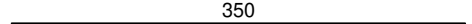
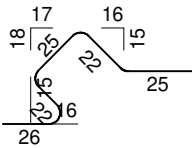
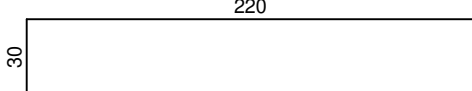
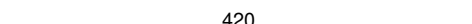
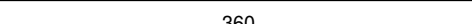
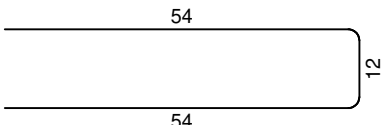
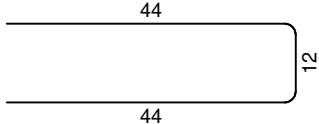
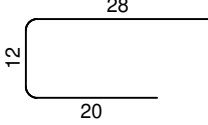
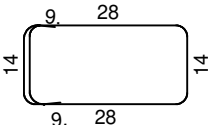
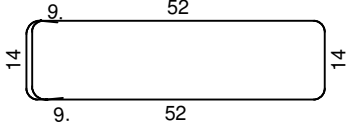
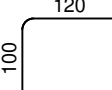
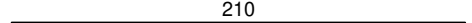
ZD-01/24
ZD-01/24-G
PZI
JULIJ 2026
2.4.1.

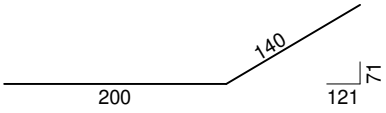
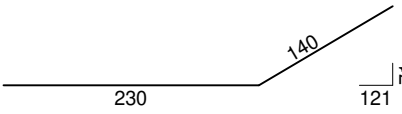
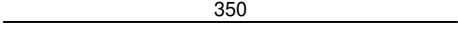
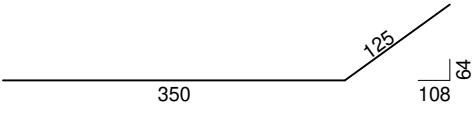
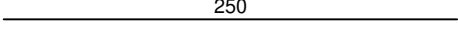
2.5. GRAFIČNE PODLOGE – ARMATURA

Armaturni načrt

[illegible]

Palice - specifikacija						
ozn	oblika in mere [cm]	Ø	lg [m]	n [kos]	lgn [m]	Opomba
Načrt AB Dvigalnega jaška in opornih zidov (List št. 01, 02) (1 kos)						
1		12	2.30	12	27.60	
2		12	2.45	12	29.40	
3		8	1.10	50	55.00	
4		8	2.00	42	84.00	
5		12	1.90	18	34.20	
6		12	2.05	18	36.90	
7		12	6.00	16	96.00	
8		12	1.50	8	12.00	
9		10	6.00	4	24.00	
10		10	1.20	2	2.40	
11		12	3.00	4	12.00	
12		10	3.00	42	126.00	
13		8	3.30	42	138.60	
14		12	1.70	20	34.00	
15		12	3.45	8	27.60	
16		10	3.45	2	6.90	
17		12	3.65	8	29.20	
18		10	3.35	2	6.70	
19		12	3.70	2	7.40	
20		10	3.70	6	22.20	
21		12	2.30	4	9.20	
22		12	1.65	8	13.20	
23		10	1.65	2	3.30	
24		10	2.00	15	30.00	

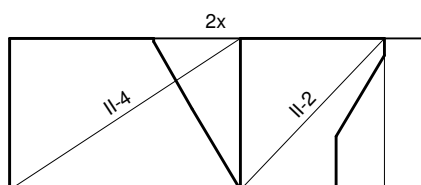
Palice - specifikacija						
ozn	oblika in mere [cm]	Ø	lg [m]	n [kos]	lg _n [m]	Opomba
25		8	2.50	75	187.50	
26		12	3.75	8	30.00	
27		10	3.75	2	7.50	
28		12	4.15	8	33.20	
29		10	4.15	2	8.30	
30		8	2.34	126	294.84	
31		12	3.50	4	14.00	
32		10	1.20	10	12.00	
33		14	2.50	34	85.00	
34		14	4.20	34	142.80	
35		14	3.60	34	122.40	
36		8	1.20	518	621.60	
37		8	1.00	79	79.00	
38		8	0.60	85	51.00	
39		8	1.16	33	38.28	
40		8	1.64	33	54.12	
41		10	1.50	55	82.50	
42		10	2.20	27	59.40	
43		14	2.10	4	8.40	

Palice - specifikacija						
ozn	oblika in mere [cm]	Ø	lg [m]	n [kos]	lgn [m]	Opomba
44		12	3.40	4	13.60	
45		12	3.70	4	14.80	
46		14	3.50	4	14.00	
47		12	4.75	4	19.00	
48		12	2.50	4	10.00	

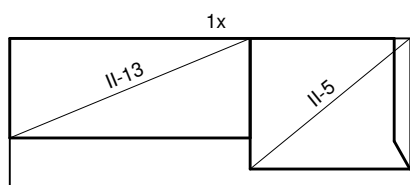
Palice - izvleček			
Ø [mm]	lgn [m]	Teža enote [kg/m']	Teža [kg]
B500B			
8	1603.94	0.41	657.62
10	391.20	0.65	254.28
12	503.30	0.92	463.04
14	372.60	1.25	465.75
Skupaj (B500B)			1840.68
Skupaj			1840.68

Mreže - specifikacija							
Pozicija	Oznaka mreže	B [cm]	L [cm]	n	Teža enote [kg/m2]	Skupna teža [kg]	Opomba
Načrt AB Dvigalnega jaška in opornih zidov (List št. 01, 02) (1 kos)							
I-1	Q-385	190	200	4	6.04	91.78	
I-2	Q-385	190	400	4	6.04	183.55	
I-3	Q-385	190	355	4	6.04	162.90	
I-4	Q-385	180	85	2	6.04	18.48	
I-5	Q-385	180	80	2	6.04	17.39	
I-6	Q-385	180	120	2	6.04	26.08	
I-7	Q-385	185	200	1	6.04	22.34	
I-8	Q-385	180	200	2	6.04	43.47	
I-9	Q-385	180	400	2	6.04	86.95	
I-10	Q-385	180	355	2	6.04	77.16	
I-29	Q-385	220	245	2	6.04	65.09	
I-30	Q-385	60	245	2	6.04	17.75	
II-1	Q-335	220	140	2	5.33	32.69	
II-2	Q-335	220	210	2	5.33	49.09	
II-4	Q-335	220	335	2	5.33	78.62	
II-5	Q-335	190	233	2	5.33	47.08	
II-9	Q-335	220	145	2	5.33	33.86	
II-10	Q-335	153	119	2	5.33	19.44	
II-11	Q-335	150	210	2	5.33	33.47	
II-12	Q-335	180	160	1	5.33	15.29	
II-13	Q-335	145	350	1	5.33	26.99	
II-14	Q-335	160	280	1	5.33	23.82	
Skupaj						1173.29	
Načrt AB Temeljne plošče (List št. 03) (1 kos)							
I-3	Q-335	220	485	4	5.33	227.30	
I-6	Q-335	75	190	1	5.33	7.59	
I-7	Q-335	220	124	1	5.33	14.47	
I-8	Q-335	220	155	1	5.33	18.16	
Skupaj						267.52	

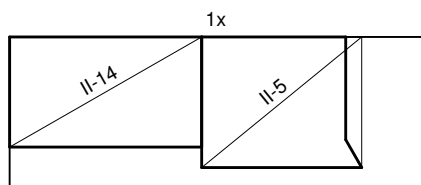
Mreže - izvleček						
Oznaka mreže	B [cm]	L [cm]	n	Teža enote [kg/m2]	Skupna teža [kg]	Neto vgrajena teža [kg]
Q-335	220	600	12	5.33	843.60	553.25
Q-385	220	600	13	6.04	1036.10	812.94
Skupaj					1879.70	1366.19



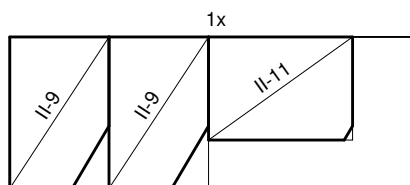
II-4 335 x 220
II-2 210 x 220



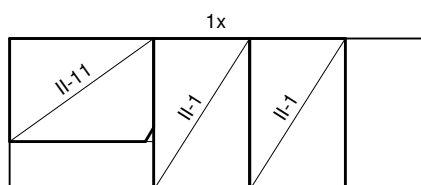
II-13 350 x 145
II-5 233 x 190



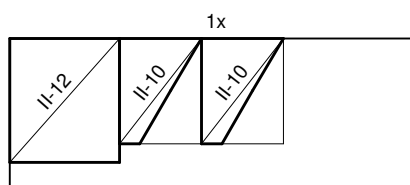
II-14 280 x 160
II-5 233 x 190



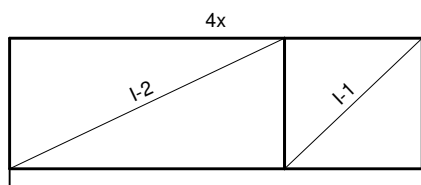
II-9 145 x 220
II-9 145 x 220
II-11 210 x 150



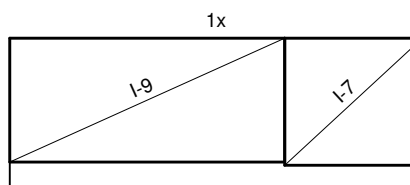
II-11 210 x 150
II-1 140 x 220
II-1 140 x 220



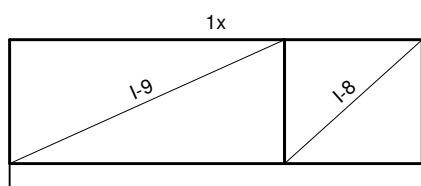
II-12 160 x 180
II-10 119 x 153
II-10 119 x 153



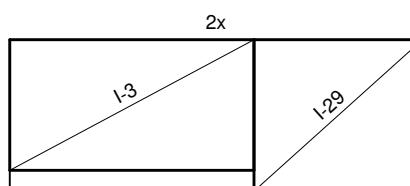
I-2 400 x 190
I-1 200 x 190



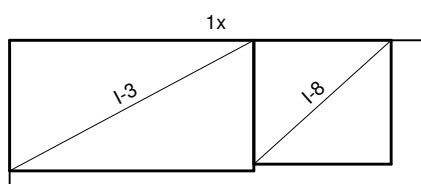
I-9 400 x 180
I-7 200 x 185



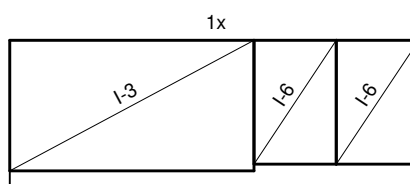
I-9 400 x 180
I-8 200 x 180



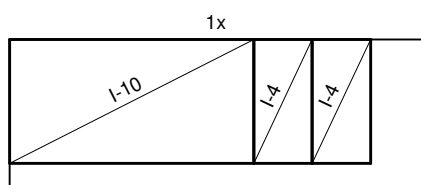
I-3 355 x 190
I-29 245 x 220



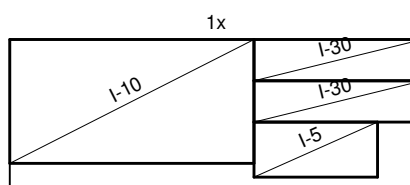
I-3 355 x 190
I-8 200 x 180



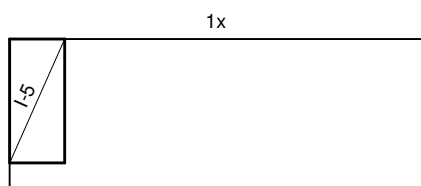
I-3 355 x 190
I-6 120 x 180
I-6 120 x 180



I-10 355 x 180
I-4 85 x 180
I-4 85 x 180

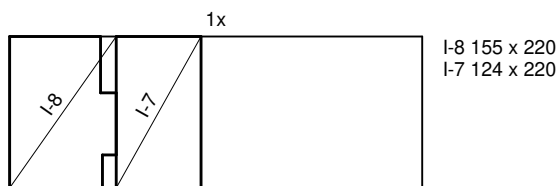
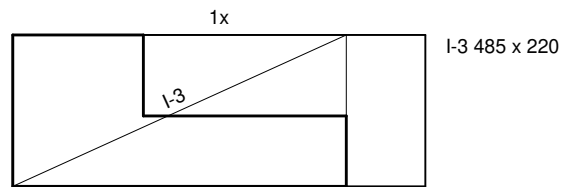
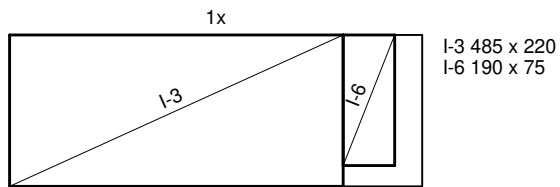
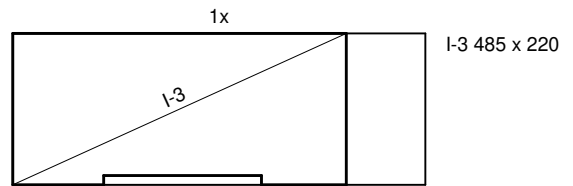
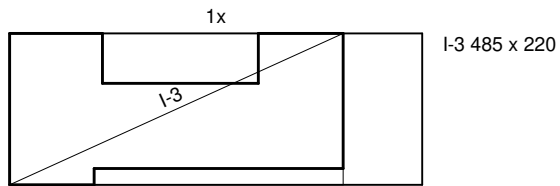


I-10 355 x 180
I-30 245 x 60
I-30 245 x 60
I-5 80 x 180

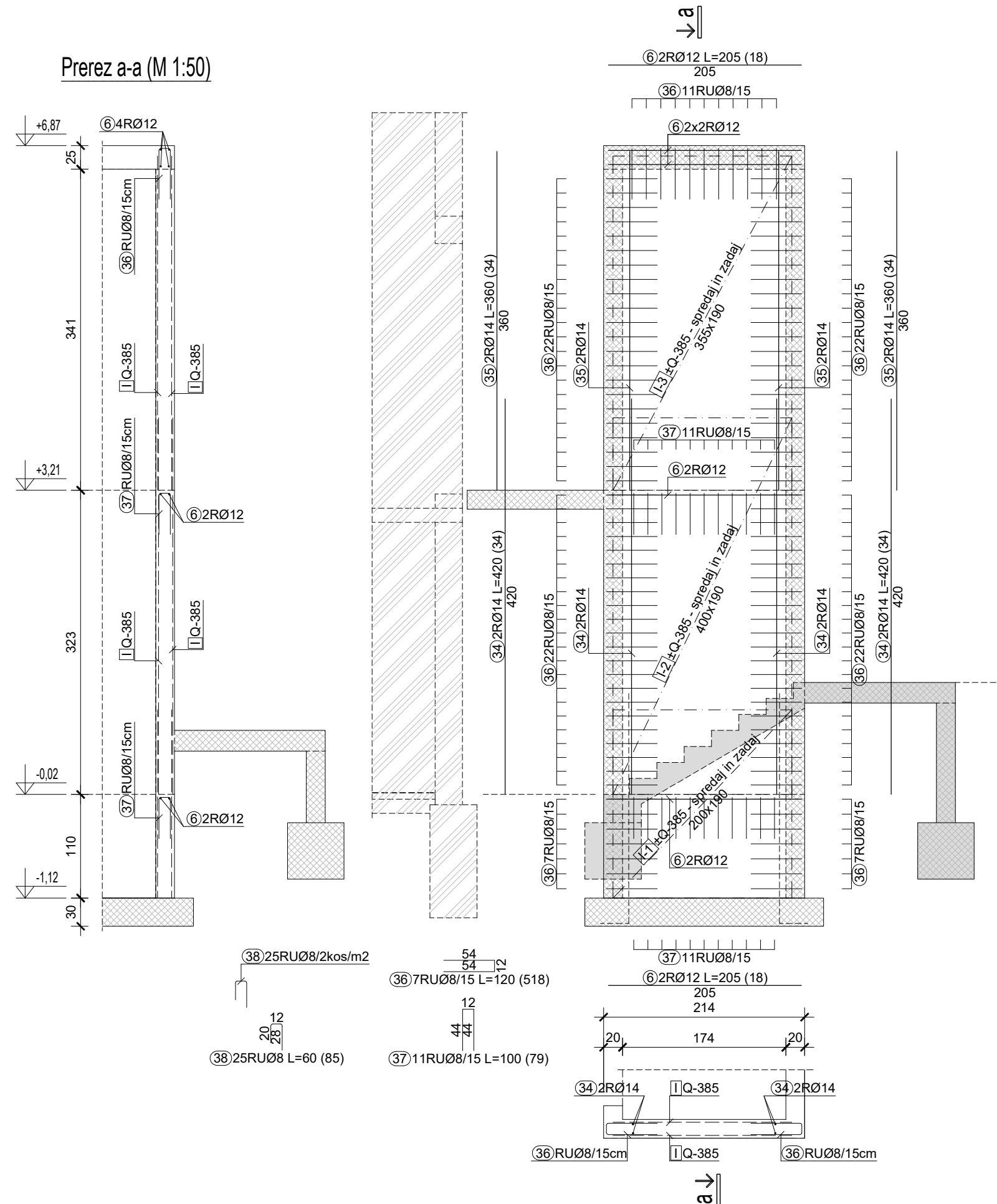


I-5 80 x 180

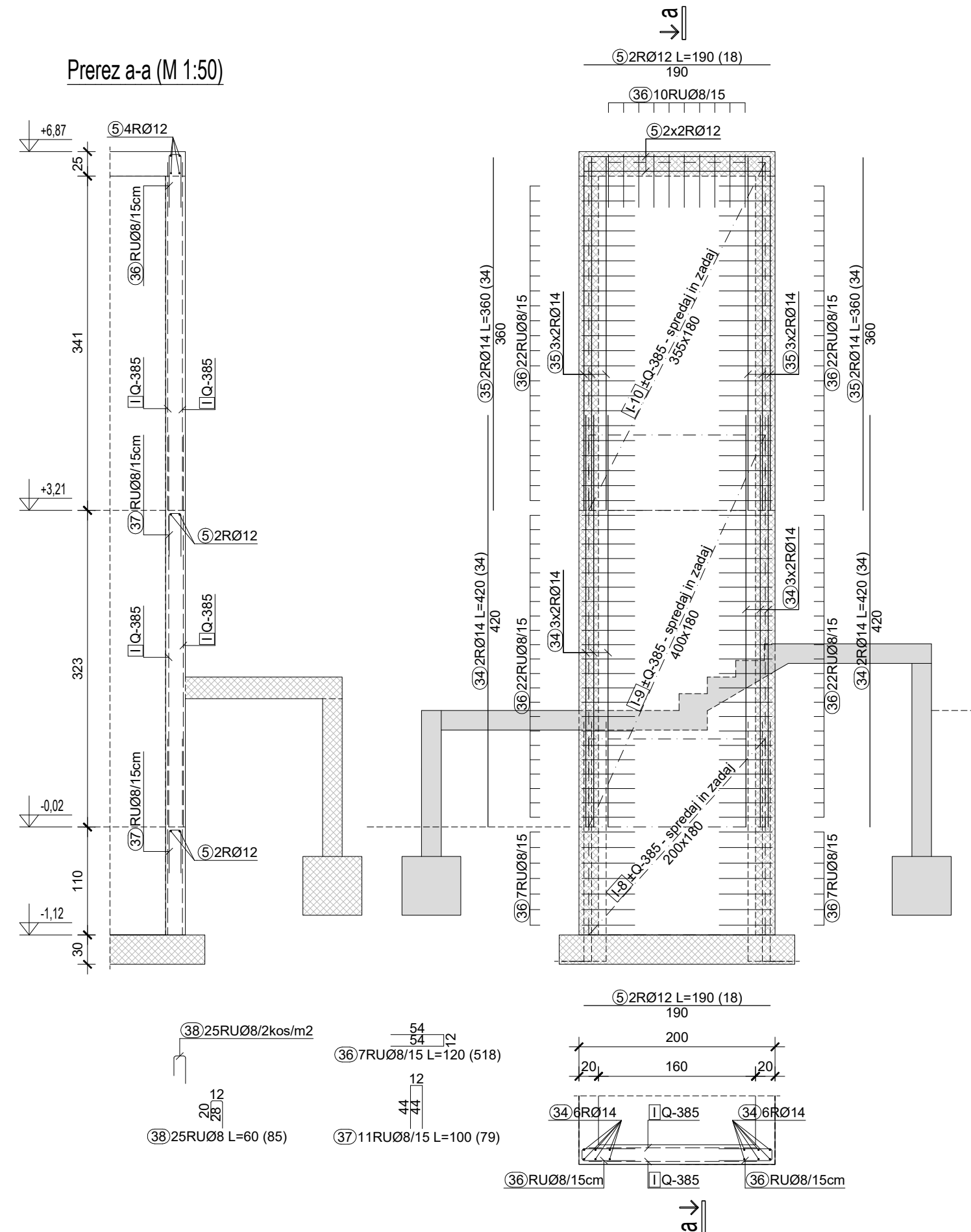
Mreže - načrt razreza
Načrt AB Temeljne plošče (List št. 03)
Q-335 (600 cm x 220 cm)



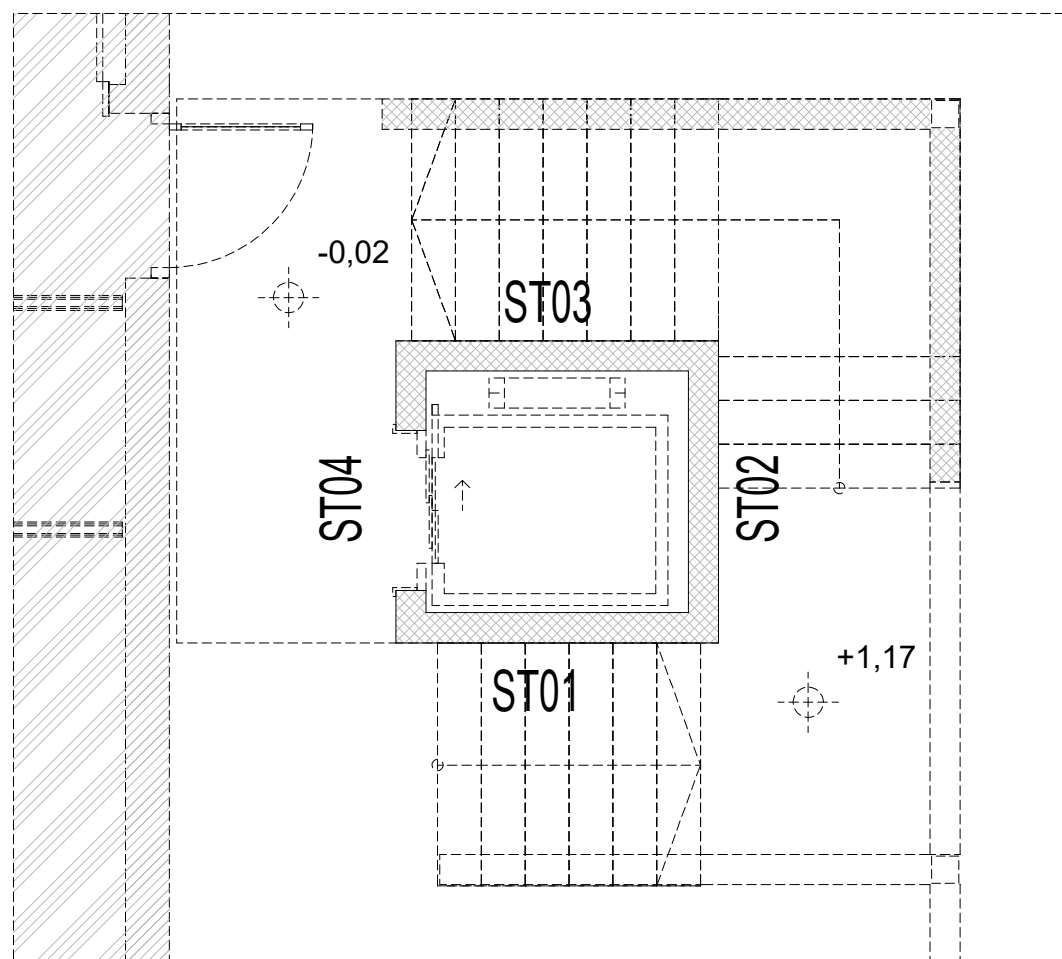
AB Stena Poz. ST01; b = 20cm
kos. 1



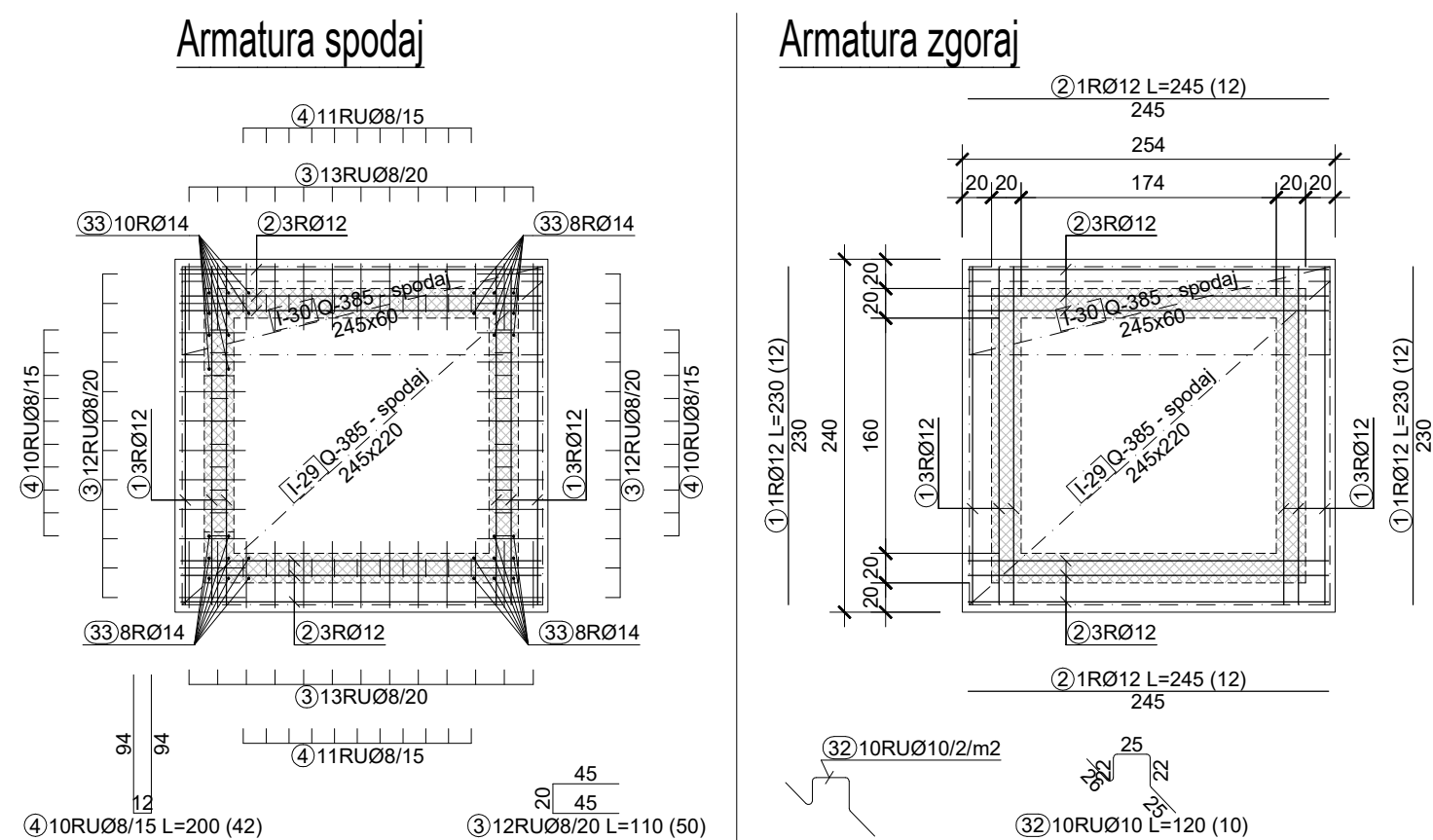
AB Stena Poz. ST02; b = 20cm
kos. 1



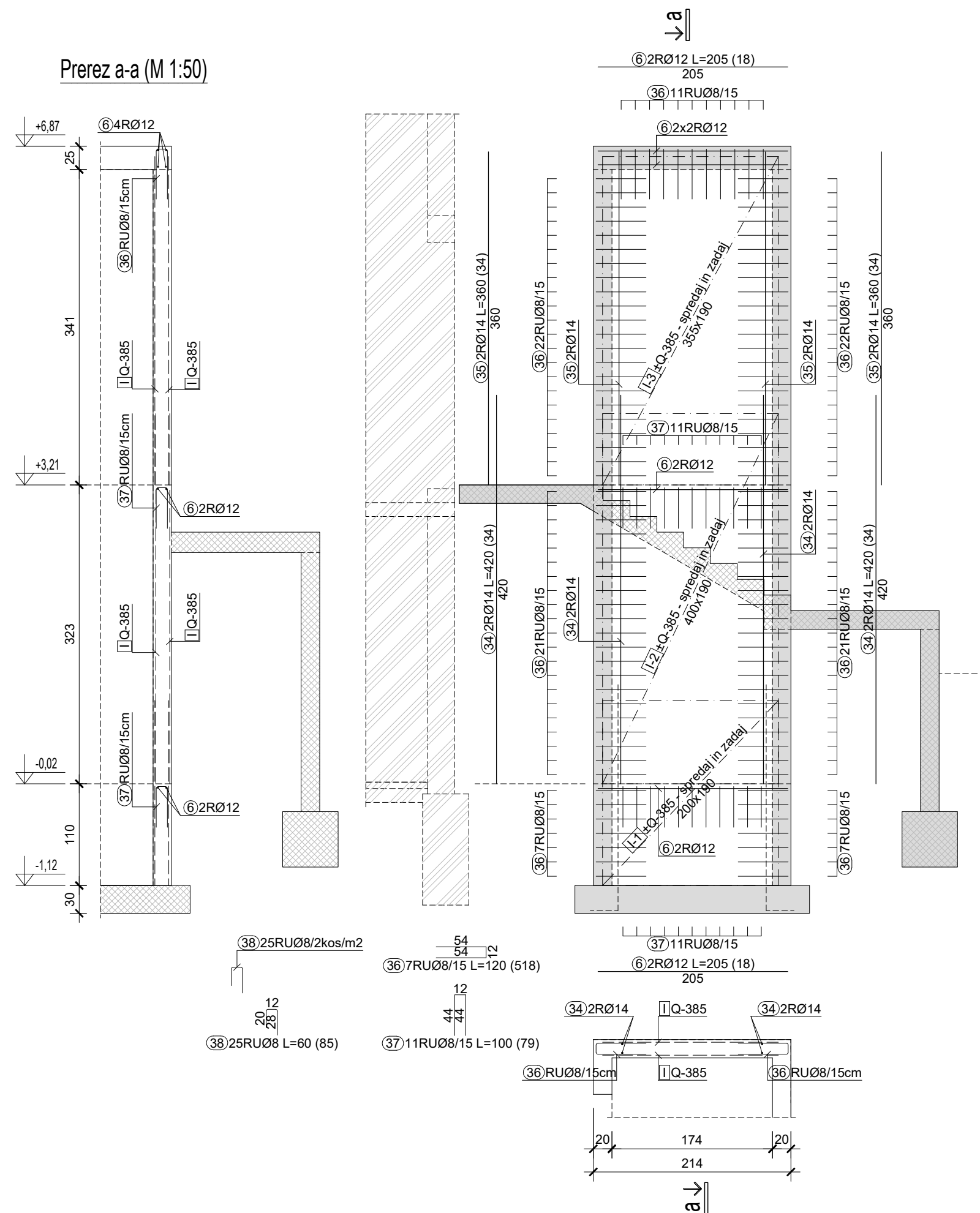
POZICIJSKI NAČRT - Dvigalni jašek



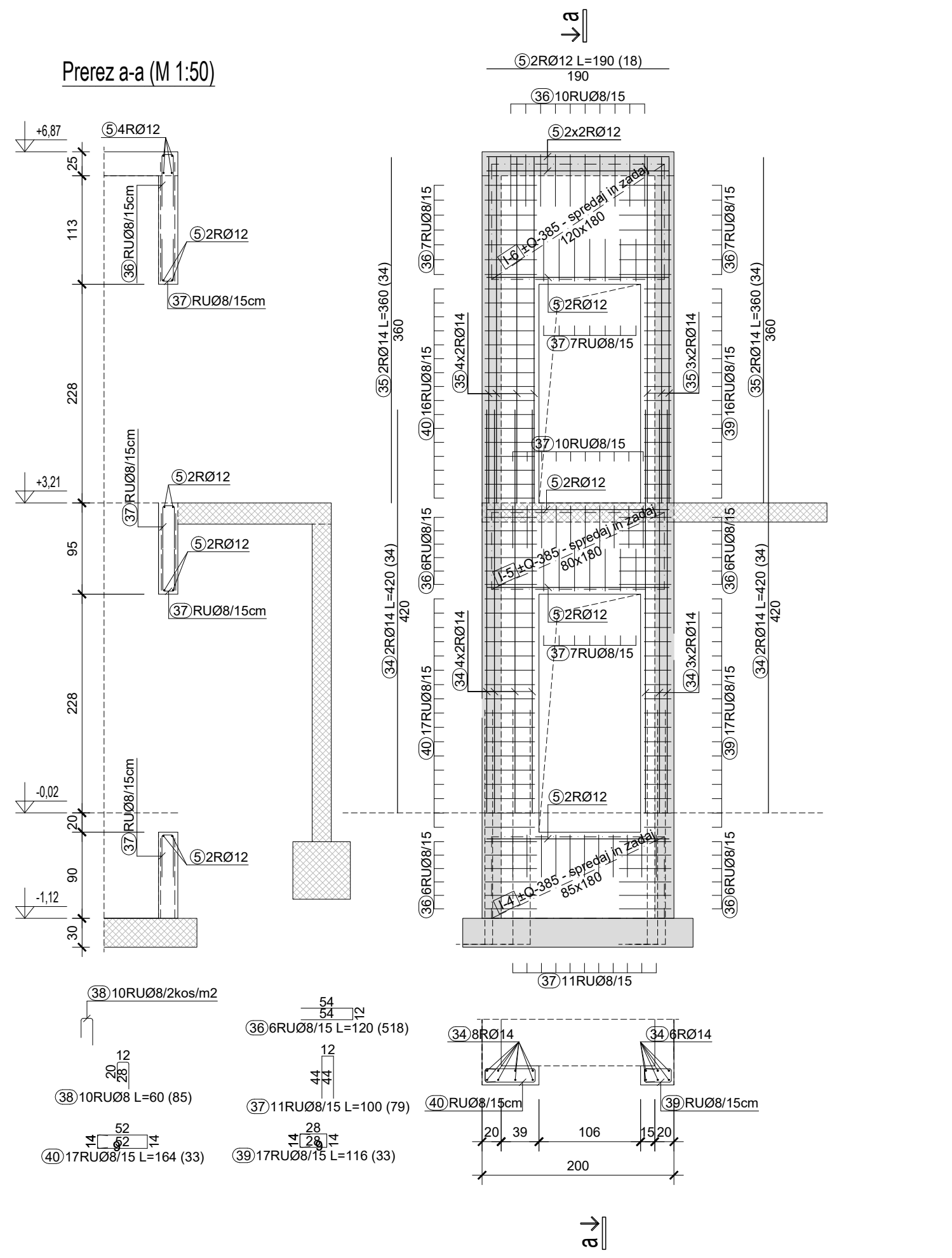
Poz. Dno dvigalnega jaška; a/b/h=254/240/30cm
kos. 1



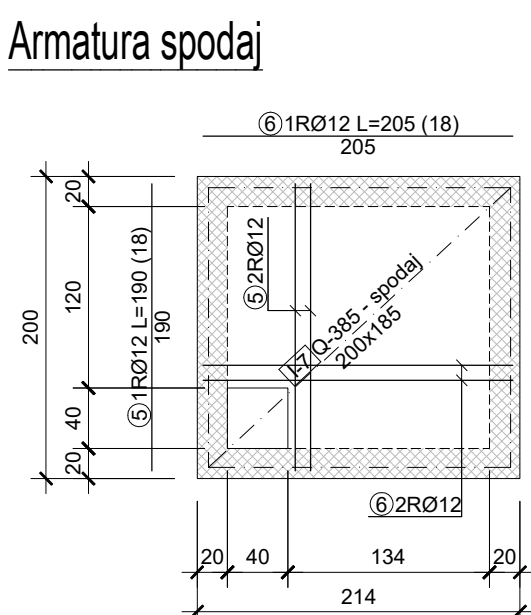
AB Stena Poz. ST03; b = 20cm
kos. 1



AB Stena Poz. ST04; b = 20cm
kos. 1

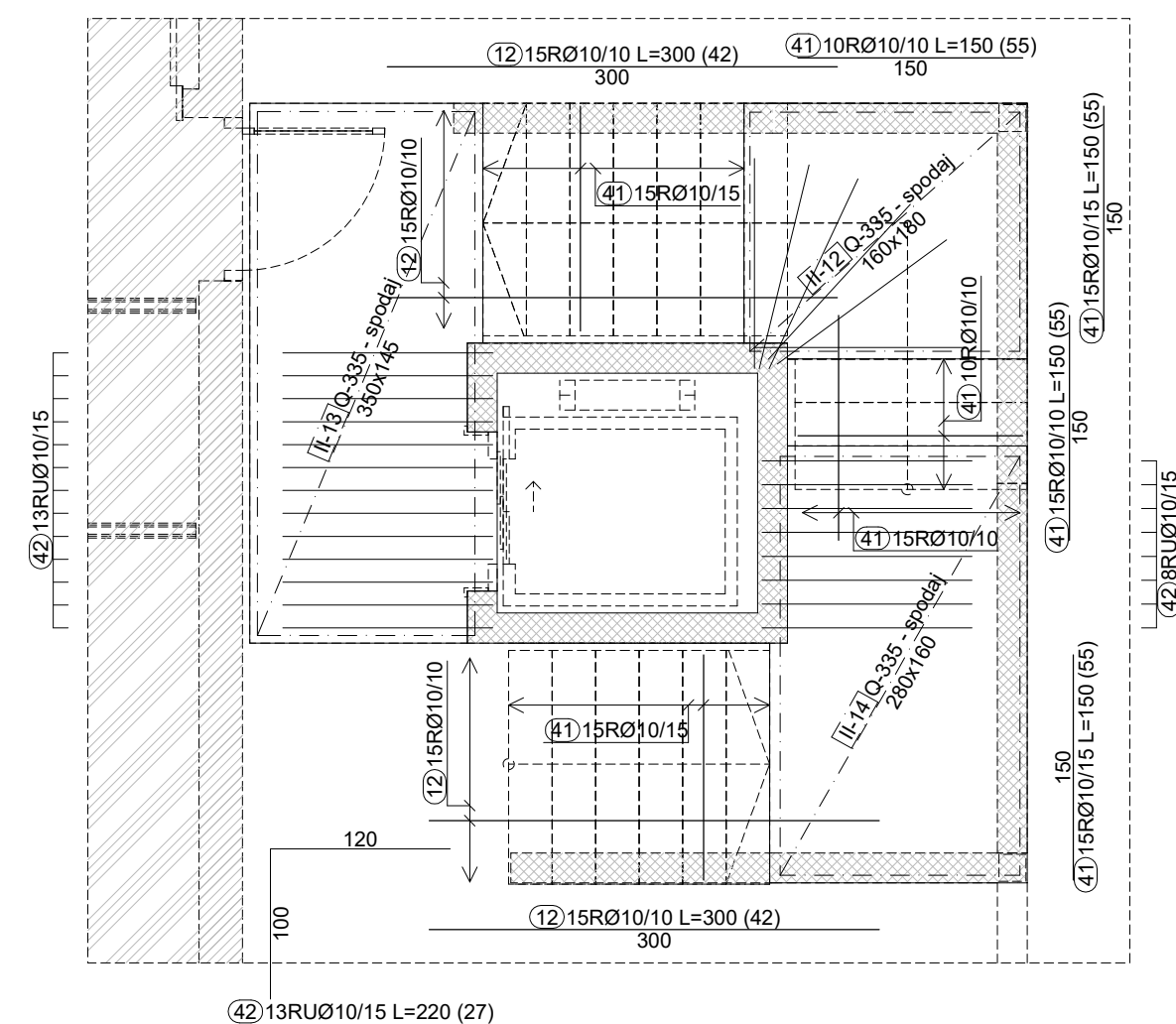


Poz. Strop dvigalnega jaška; a/b/h=254/240/25cm
kos. 1



AB Stopnice dvigalnega jaška

Armaturu prilagodi dejanskemu stanju na gradbišču!!!



PODATKI O MATERIALIH										
BETON	Lestnica / valduk / S1ST EN 206/2013, S1ST EN 1026/2016 betonski valduk / S1ST EN 13697/2010 A10/1/2010	Specifikacije projektnog betona						Zaštitni sloj [cm]		
		Temperaturni rasponi [N/mm²]	Razred opstojljivosti, stonj vlačnosti klondov, maksimalno zmo agregata [mm], razred konsistencije, dodatke						agregat	betonski valduk
PODLOŽNI BETON		C12/15	X0	CL 0.2	D _{max} 16	S1				
TEMELJNA PLOŠĆA, TEMELJI		C25/30	XC1	CL 0.2	D _{max} 16	S4		4,0	4,0	4,0
ETAŽNE PLOŠĆE / NOSILICI		C25/30	XC2	CL 0.2	D _{max} 16	S4		3,5	2,5	2,5
STENE		C25/30	XC4/XF1	CL 0.2	D _{max} 16	S4		2,0	3,0	3,0
VEŽI / STEBRI / STOPNICE		C25/30	XC2	CL 0.2	D _{max} 16	S4		2,5	2,5	2,5
PROJEKT BETONA IZDELA DOBAVITELJ SVJEŽE BETONSKE MEŠANICE !!										

ARMATURNO JEKLO Lastnosti v skladu s SIST EN 10082: 2005 Krivljenje v skladu s SIST EN 1992-1-1	Vrsta jekla	Trdnostni razred (N/mm ²)	Stopnja duktilnosti (%)
REBRASTE ARMATURNE MREŽE	S 500 A	500	srednja ≥ 2,5
REBRASTE ARMATURNE PALICE	S 500 B	500	visoka ≥ 5,0

OPOMBE:

- V skladu s predpili, mora izvajalec: del pred začetkom izvajanja del, opraviti pregled projekta za izvedbo (PZI) in opozoriti na morebitne ugotovljene pomanjkljivosti in zahteva njihovo odpravo.
- **IZJAVILEC DEL JAMČI ZA PRAVILNOST IZVEDBE**
- Mere, vsitnine kolc in preboje preveriti v oštebilih načrtih, ter jih prilagoditi dejanskemu stanju.
- Pred vgradnjo betona vgrajeno armaturo obvezno preveriti odgovorni nadzornik za gradbeni dela. Položeno armaturo pregledati z vpisom v gradbeni dnevnik. Polaganje armature se prilagodi v skladu z morebitnimi spremembam v arhitekturnih načrtih PZI!
- Preboji manjši od Ø30 cm niso risani v načrtu armature. Armatura se reže na lica mesta po obodu preboja.
- Preklop mreže je 40cm oz vaj 3 okenca. Na mestih kjer se mreže prekrivajo je mreže potrebno obrniti tako, da se mreže uležajo ena v drugo.

Datum:	Opis sprememb:	Potrdil:	Podpis:
Spremembe:			

**ZAS
NOV
A** GRADBENI BIRO
Lovšin Tone s.p.
Vrvarska pot 1a
1310 Ribnica, Slovenija

NAČRT AB DVIGALNEGA JAŠKA

sestavni del: 2. Načrt gradbeništva
 objekt: PRENOVA ZDRAVSTVENE POSTAJE VELIKE LAŠČE
 vodja projekta: Miha Novina, univ. dipl. inž. arh., ZAPS A 1654
 pooblaščen inž.: Tone Lovšin, dipl. inž. grad., IZS G -0379
 sodelavec: Sandi Mohar, dipl.inž.grad., IZS G - 4126
 Dejan Kastelec, dipl.inž.grad., mag.stavb.

M 1:50 / 1:25

št. projekta:	ZD-01/24-
št. načrta:	ZD-01/24-0
vrsta projekta:	PZ
datum:	Julij 2024
št. lista:	2.5.

[illegible]

Technical drawing of a rectangular plate with the following specifications:

- Overall width: 60
- Overall height: 140
- Top edge features three holes:
 - Left hole: 11.2RØ12, 20 from left edge, 20 from top edge.
 - Middle hole: 2.2RØ10, 20 from left edge, 20 from top edge.
 - Right hole: 2.2RØ10, 20 from right edge, 20 from top edge.
- Bottom edge features two holes:
 - Left hole: 7.4RØ12, 20 from left edge, 140 from top edge.
 - Right hole: 7.4RØ12, 20 from right edge, 140 from top edge.
- Internal features:
 - A dashed rectangle in the center, 60 wide and 80 high, with a 20mm offset from the top and bottom edges.
 - A central hole: Ø2RØ10, 60 from left edge, 80 from bottom edge.
- Surface finish: 3.0/1.6 Ra (indicated on the left side).
- Dimensional tolerances:
 - ±0.57 for the top edge.
 - ±0.02 for the top edge of the central hole.
 - ±0.32 for the bottom edge of the central hole.
 - ±0.02 for the bottom edge.

[illegible]

Technical drawing of a staircase showing two views: a plan view (top) and a side elevation view (bottom). The plan view shows a rectangular staircase with dimensions 155 (width) and 175 (depth). It includes a landing area with dimensions 60 and 115. The side elevation view shows the staircase profile with a total height of 165 and a depth of 175. It includes a landing area with dimensions 60 and 115. The drawing is labeled "Prerez a-a (M 1:25)".

Technical drawing of a building section showing structural elements and dimensions. The drawing includes a plan view at the top and a section view below it.

Plan View (Top):

- Overall width: 300
- Overall depth: 300
- Internal width dimension: 294
- Internal depth dimension: 20,20
- Structural elements labeled:
 - (11) 2RØ12 L=300 (4)
 - (12) 2RØ10 L=300 (42)
 - (13) 14RUØ8/20
 - (14) 2RØ10
 - (15) 4RØ12
 - (16) 2RØ10
 - (17) 2RØ10
 - (18) 2RØ10 L=345 (2)
 - (19) 30/12RUØ8/20
 - (20) 12RUØ20/m
 - (21) 345
 - (22) 345

Section View (Bottom):

- Overall height: 14,2
- Overall width: 300
- Internal width dimension: 294
- Internal height dimension: 14,2
- Structural elements labeled:
 - (11) 2RØ12
 - (12) 2RØ10
 - (13) 14RUØ8/20
 - (14) 2RØ10
 - (15) 4RØ12
 - (16) 2RØ10
 - (17) 2RØ10
 - (18) 2RØ10 L=345 (2)
 - (19) 30/12RUØ8/20
 - (20) 12RUØ20/m
 - (21) 345
 - (22) 345

[illegible]

Technical drawing of a rectangular plate with the following dimensions and annotations:

- Overall width: 60
- Overall height: 277
- Top edge: 44x4RØ12 (radius 12)
- Left edge: 38 RUØ8/15cm (radius 8, length 15cm)
- Right edge: 27 RUØ8/15cm (radius 8, length 15cm)
- Bottom edge: 7x4RØ12 (radius 12)
- Internal dimensions: 20 (width of top and bottom flanges), 60 (width of central section), 217 (height of central section), 60 (width of bottom flange)
- Annotations: 44x4RØ12, 38 RUØ8/15cm, 27 RUØ8/15cm, 7x4RØ12, 8x2RØ10 (radius 10), 0.02, 0.32, 0.52

Technical drawing of a staircase section showing structural details and reinforcement. The drawing includes a side elevation of the staircase with a landing, showing the concrete structure and the placement of reinforcement bars. Key dimensions and reinforcement specifications are labeled: overall width 223, landing width 125, and total width 160. Reinforcement bars are specified as 47#2xR012, 36#14RU08/15, 47#2R012 L=415 (1), 36#8RU08/15, 48#2xR012, 13#2xR012, 48#2xR014, 36#14RU08/15, 14#4x0.55 250x250, 15#4x0.55 250x250, 28#4R012, 22#2R010, 28#4R012, 30#16RU08/20, 25#25RU08/15, 422, 60, 22#2R010 L=415 (2), 415, 28#4R012 L=415 (3), and 415. A north arrow is present in the top right corner.

[illegible]

PODATKI O MATERIÁLH		Specifikácie projektnej betóna		Základní šířky (cm)			
BETON Rozsah: 10000 m ³ (DST EN 206:2012), 1000 m ³ (DST EN 1008:2012) Vek betónu: 28 dní (1.367.202.10A.10.2012)		Tabuľka	Rozsah projektnej betóna Rozsah: 10000 m ³ (DST EN 206:2012), 1000 m ³ (DST EN 1008:2012) Vek betónu: 28 dní (1.367.202.10A.10.2012)	Šírka základu	Šírka základu	Šírka základu	Šírka základu
		Tabuľka	Rozsah projektnej betóna Rozsah: 10000 m ³ (DST EN 206:2012), 1000 m ³ (DST EN 1008:2012) Vek betónu: 28 dní (1.367.202.10A.10.2012)	Šírka základu	Šírka základu	Šírka základu	Šírka základu
POLOŽENIE BETONU		X0	CL 0,2	D 0,2	D 0,2	D 0,2	D 0,2
TEMEĽNÁ PLOŠA, TEMELI	C25/50	X01	CL 0,2	D 0,2	D 0,2	D 0,2	D 0,2
ETAŽNÉ PLOŠE / NOSIČI	C25/50	X02	CL 0,2	D 0,2	D 0,2	D 0,2	D 0,2
STĚNY	C25/50	X04X01	CL 0,2	D 0,2	D 0,2	D 0,2	D 0,2
VEŽI / STĚBY / STUPNICE	C25/50	X05	CL 0,2	D 0,2	D 0,2	D 0,2	D 0,2

PROJEKT BETONU (ZDĚLA DODAVATEL SVÉHO BETONOVÉHO MĚŠÁNÍ !!)

ARMATURNO JEKLO (Lastnosti v skladu s SIST EN 10080:2005 Krivljeno v skladu s SIST EN 1992-1-1	Vrsta jekla	Trdotnostni razred (N/mm ²)	Stopnja dolžinosti [%]
REBRASTE ARMATURNE MREŽE	S 500 A	500	srednja ≥ 2,5
REBRASTE ARMATURNE PALICE	S 500 B	500	visoka ≥ 5,0

OPOMBE:

- V skladu s predpisi, mora izvajalec del pred začetkom izvajanja del, oporiti pregled projekta za izvedbo (PZI) in opozoriti na morebitne upoštevne pomankljivosti in zahteva njihovo odpravo.
- IZJAVILEC DEL JAMČI ZA PRAVILNOST IZVEDBE
- Mere, vršinske kote in preboje preveriti v obehih nadstropjih, ter jih prilagoditi dejanskemu stanju.
- Pred vsakim betonskim prelivom in pred vsakim oporo preveriti odstopanje nadstropij za gradbeno delo. Poročeno stanje oporo preveriti v vsaki izvedbi gradbenih delov. Poligiranje armature se prilagodi v skladu z morebitnimi spremembami v arhitekturnih načrtih PZI.
- Preboji manj od 330 cm niso risani v načrtu armature. Armature se ne loči na licu mesta od oboje preboja.
- Preklopi merje je 40cm oz. vaj 3 okenca. Na mestih kjer se mreže prekrivajo je mreže potrebno obrniti tako, da se mreže uležajo ena v drugo.

Datum:	Opis spremembe:	Potrdil:	Podpis:
Sprememba:			

**ZAS
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A** GRADBENI BIRO
Lovšin Tone s.p.
Vrvarska pot 1a
1310 Ribnica, Slovenija

NAČRT AB OPORNIH ZIDOV

2. Način gradbeništva
PRENOVA ZDRAVSTVENE POSTAJE VELIKE LAŠE
Miha Novina, univ. dipl. inž. arh., ZAPS A 1654
Tone Lovšin, dipl. inž. grad., IZS G-0379
Sandi Mohar, dipl. inž. grad., IZS G- 4126
Dejan Kastelec, dipl. inž. grad., mag. stavb.

M 1:50 / 1:2

projekta:	ZD-01/
načta:	ZD-01/24
projekta:	F
datum:	Julij 20
št. list:	2.5



PODATKI O MATERIALIH									
BETON Lastnosti v skladu s SIST EN 206:2013, SIST EN 1026:2016 Izdelava v skladu s SIST EN 13670:2010/A101:2010	Specifikacije projektnega betona						Zaščitni sloj [cm]		
	Trdnostni razred [N/mm²]	Razred izpostavljenosti, stonje vsabnosti kloridov, maksimalno zmo agregate (mm), razred konsistence, dodatno					zgornja, zunanja	spodnja, notranja	bočna, zasluga
PODLOŽNI BETON	C12/15	X0	CL 0,2	D _{max} 16	S1				
TEMELJNA PLOŠČA, TEMELJI	C25/30	XC1	CL 0,2	D _{max} 16	S4		4,0	4,0	4,0
ETAŽNE PLOŠČE / NOSILCI	C25/30	XC2	CL 0,2	D _{max} 16	S4		2,5	2,5	2,5
STENE	C25/30	XC4/XF1	CL 0,2	D _{max} 16	S4		3,0	3,0	3,0
VEZI / STEBRI / STOPNICE	C25/30	XC2	CL 0,2	D _{max} 16	S4		2,5	2,5	2,5
PROJEKT BETONA IZDELA DOBAVITEL SVEŽE BETONSKE MEŠANICE !!									

ARMATURNO JEKLO Lastnosti v skladu s SIST EN 10080; 2005 Krivljeno v skladu s SIST EN 1992-1-1	Vrsta jekla	Trdnostni razred [N/mm²]	Stopnja duktilnosti [%]
REBRASTE ARMATURNE MREŽE	S 500 A	500	srednja ≥ 2,5
REBRASTE ARMATURNE PALICE	S 500 B	500	visoka ≥ 5,0

- OPOMBE:
- V skladu s predipi. mora izvajalec del pred začetkom izvajanja del, opraviti pregled projekta za izvedbo (PZI) in opozoriti na morebitne ugotovljene pomanjkljivosti in zahteva njihovo odpravo.
 - IZVJALEC DEL JAMČI ZA PRAVILNOST IZVEDBE
 - Mere, višinske kote in preboje preveriti v osatlih načrtih, ter jih prilagoditi dejanskemu stanju.
 - Pred vgradnjo betonov vgrajeno armaturo obvezno preveriti odgovorni nadzornik za gradbena dela. Položeno armaturo potrdi z vpisom v gradbeni dnevnik. Polaganje armature se prilagodi v skladu z morebitnimi spremembami v arhitekturnih načrtih PZI !
 - Preboji manjši od Ø30 cm niso risani v načrtu armature. Armaturo se reže na licu mesta po obodu preboja.
 - Preklop mrež je 40cm oz vaj 3 okenca. Na mestih kjer se mreže prekrivajo je mreže potrebno obrniti tako, da se mreže uležejo ena v drugo.

Datum:	Opis sprememb:	Potrdil:	Podpis:
Spremembe:			

ZASNOVA

GRADBENI BIRO
Lovšin Tone s.p.
Vrvarska pot 1a
1310 Ribnica, Slovenija

NAČRT AB TALNE PLOŠČE

sestavni del: 2. Načrt gradbeništva
objekt: PRENOVA ZDRAVSTVENE POSTAJE VELIKE LAŠČE
vodja projekta: Miha Novina, univ. dipl. inž. arh., ZAPS A 1654
poodlašeni inž.: Tone Lovšin, dipl. inž. grad., IZS G -0379
sodelavec: Sandi Mohar, dipl.inž.grad., IZS G - 4126
Dejan Kastelec, dipl.inž.grad., mag.stavb.

M 1:50 / 1:25
št. projekta: ZD-01/24
št. načrta: ZD-01/24-G
faza projekta: PZI
datum: Julij 2026
št. lista: 2.5.3